∞ ∞ Š \mathcal{H} ENCEFER

2

94 446 Œ PRO SEE SHEET 3 FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

<u>LINE</u>	STATION	<u>PLAN</u>	PROFILE
-L-	11+80 - 31+94	4-5	6
-Y2-	11+60 - 12+86	5	6
-Y3-	11+13 - 14+16	5	-

CROSS SECTIONS

LINE	STATION	SHEETS
-L-	II+50 - 32+00	7-32
-Y2-	11+50 - 12+50	33
-Y3-	11+00 - 14+00	34-35

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

ROADWAY SUBSURFACE INVESTIGATION

COUNTY **ONSLOW**

PROJECT DESCRIPTION EXTENSION OF COMMERCE ROAD FROM FAIRWAY ROAD TO PINEY GREEN ROAD (SR 1406) AT COUNTRY CLUB DRIVE

INVENTORY

STATE PROJECT REFERENCE NO. 35 U-5878

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1919 707-650. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

CENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABBRATORY SAMPLE DATA AND THE IN SITU (INP-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS NIDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE THE SUBSURFACE INVESTIGATION AS HE DEEMS NECESSARY TO SATISTY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR IS ALL HAVE NO CLAIM FOR ADDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY RESAON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

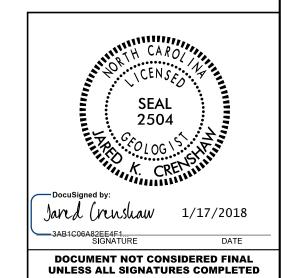
J.K. CRENSHAW CATLIN INC.

PERSONNEL

DRAWN BY __J.K. CRENSHAW CHECKED BY **B.D. KEANEY**

SUBMITTED BY _B.D. KEANEY

DATE JANUARY 2018



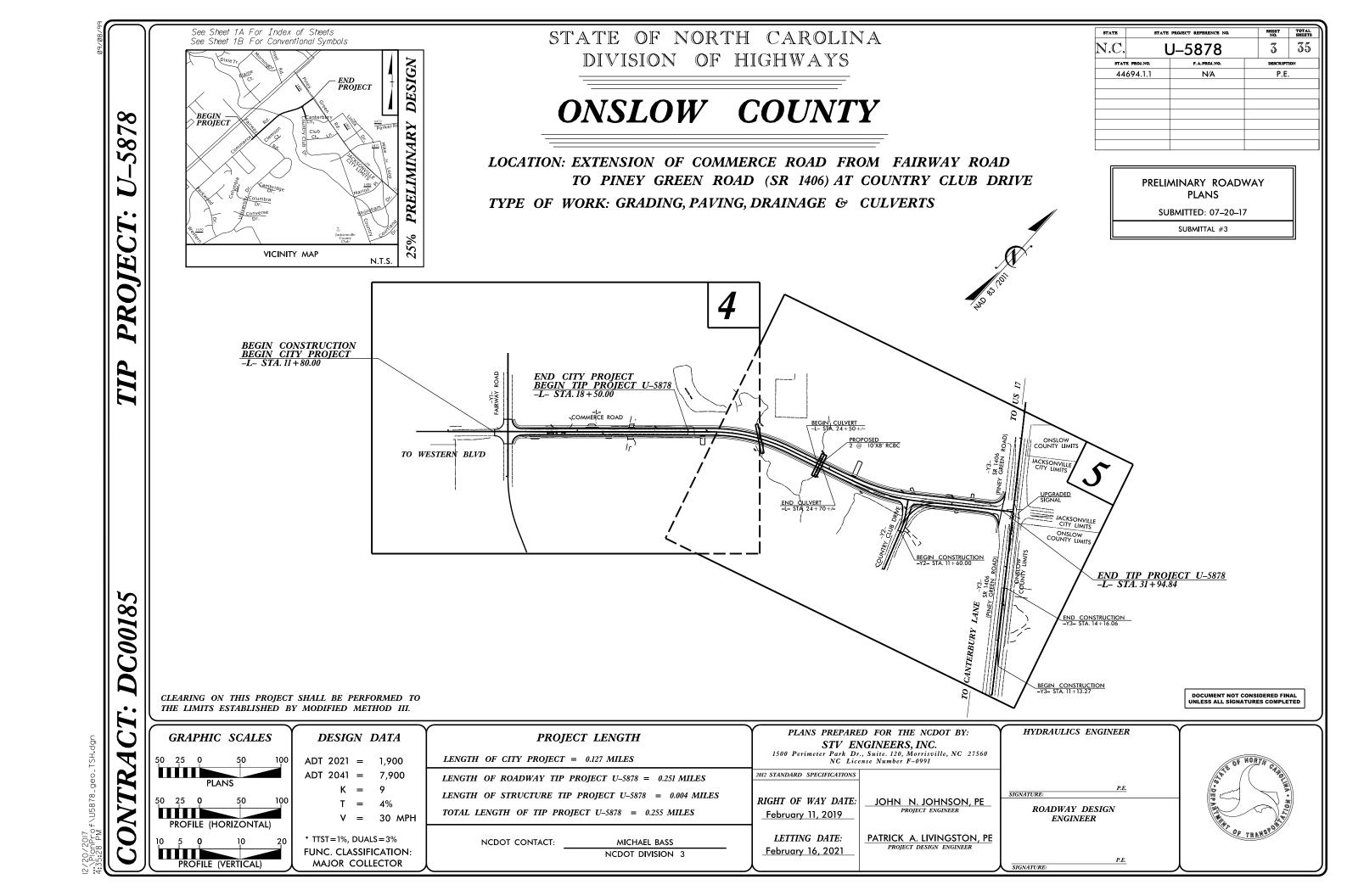
U-5878 SHEET NO.

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	<u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN Ø.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAID LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
CENERAL CRANIII AR MATERIALS SILT-CLAY MATERIALS	MINERALOGICAL COMPOSITION	FINE TO COARSE CRAIN IGNEOUS AND METAMORPHIC POCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE CEVEL HI
CLASS. (\$35% PASSING *200) (>35% PASSING *200) ORGANIC MATERIALS	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.	CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.
GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. COMPRESSIBILITY	NON CRYCTALLING FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CLASS. A-1-a A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7	SLIGHTLY COMPRESSIBLE LL < 31	ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
SYMBOL 0000d0000d	MODERATELY COMPRESSIBLE LL = 31 - 50	COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED
7. PASSING	HIGHLY COMPRESSIBLE LL > 50 PERCENTAGE OF MATERIAL	SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
*40 30 MX 50 MX 51 MN SOILS CLLAY PEAT		- WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
"2000 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN	ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE
MATERIAL PASSING *40	LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE. VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	HORIZONTAL.
LL - 40 MX 41 MN 11TH F OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE	(V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
P1 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN 10 MX 11 MN 1	GROUND WATER	OF A CRYSTALLINE NATURE.	FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC SOILS		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
USUAL TYPES STONE FRAGS. OF MAJOR GRAVEL, AND SAND GRAVEL AND SAND GRAVEL AND SAND SOILS SOILS SOLUTION OF MAJOR GRAVEL AND SAND SAND SOILS SOILS	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SAND SHIND GRAVEL HIND SHIND SUILS SUILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY, ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN. RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABL		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	→ SPRING OR SEEP	WITH FRESH ROCK.	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.
COMPACTNESS OF RANGE OF STANDARD RANGE OF UNCONFINED	III 25/025	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES 'CLUNK' SOUND WHEN STRUCK.	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE CONFIDENCY PENETRATION RESISTENCE COMPRESSIVE STRENGTH (N-VALUE) (TONS/FT ²)	ROADWAY EMBANKMENT (RE) 25/825 DIP & DIP DIRECTION WITH SOIL DESCRIPTION OF ROCK STRUCTURES	IF TESTED, WOULD YIELD SPT REFUSAL SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
GENERALLY VERY LOOSE < 4	SOIL SYMBOL SPT TEST BORING SLOPE INDICATOR	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
CRANULAR	VST PMT UNSTALLATION	TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN, IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS
MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
VERT DENSE / 30		SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK (V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
VERY SOFT < 2 < 0.25 GENERALLY SOFT 2 TO 4 0.25 TO 0.5	- INFERRED SOIL BOUNDARY - CORE BORING SOUNDING ROD	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MWONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL STIFF 8 TO 15 1 TO 2	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER SPT N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4	INSTRUCTION	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNDERCUT UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND
COARSE FINE	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
BOULDER COBBLE GRAVEL SAND SAND (SL) (CL)		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK, GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT
(CSE, SU.) (F SU.)	ABBREVIATIONS	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.005 0.005 SIZE IN. 12 3	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL
	\perp CL CLAY MOD MODERATELY γ - UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CHARGE PROPERTIES OF SCRIPTION	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{\sf d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK. SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY
(ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT.) FROM BELOW THE GROUND WATER TABLE	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC SEMISOLID: REQUIRES DRYING TO	FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE < - WEI - (W) ATTAIN OPTIMUM MOISTURE	FRAGS FRAGMENTS ω - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: SEE NOTES BELOW
PL L _ PLASTIC LIMIT	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	
OM _ OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	EQUIPMENT USED ON SUBJECT PROJECT DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	ELEVATION:FEET
SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: X	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NOTES:
- DRY - (D) REQUIRES ADDITIONAL WATER TO	6° CONTINUOUS ELIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	BL-3 ELEV=30:55' - USED AS A SURVEY REFERENCE FOR CULVILIT
ATTAIN UPTIMUM MUISTURE	CME-55	THINLY LAMINATED < 0.008 FEET INDURATION	4
PLASTICITY			BL-5 ELEV=32,39' - USED AS A SURVEY REFERENCE FOR CULV2_LT AND CULV2_RT
PLASTICITY INDEX (PI) ORY STRENGTH	X CME-550	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. RUBBING WITH FINGER FREES NUMEROUS GRAINS:	
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST VANE SHEAR TEST HAND TOOLS:	FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	ALL OTHER BORING ELEVATIONS AND GROUND ELEVATIONS OBTAINED FROM 'U5878_is_tin.tin' dated 5/2/2017
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	X CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	FIAD - FILLED IMMEDIATELY AFTER DRILLING
COLOR	PORTABLE HOIST X TRICONE 2 15/6 STEEL TEETH X HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
CULUR	TRICONE TUNGCARB SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	CORE BIT VANE SHEAR TEST	CHARD HAMMED BLOWC DECLIDED TO BREAK CAMPLE.	
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14





December 20, 2017

STATE PROJECT: 44694.1.1

TIP NUMBER: U-5878

COUNTY: Onslow

DESCRIPTION: Extension of Commerce Road from Fairway Road to Piney Green Road

(SR 1406) at Country Club Drive

SUBJECT: Geotechnical Roadway Inventory report

PROJECT DESCRIPTION

The U-5878 project is designed to improve traffic flow and ease congestion in the City of Jacksonville, NC. The project consists of extending Commerce Road to Piney Green Road. Included in this report are borings related to a roadway extension, new alignments, and two proposed culverts.

The field investigation was conducted in November of 2017 using an ATV mounted CME 550 and a track mounted CME 45B, both with automatic hammers. Standard Penetration Tests (SPT) were performed at selected locations. Borings were advanced with hollow stem augers and mud rotary equipment along the project corridor. Representative soil samples were collected and forwarded to an approved testing facility for soil quality analysis, moisture content, and AASHTO classification.

The following alignments were investigated

Line	S	tation		Length (ft)
-L-	11+80	to	31+94	2,015
-Y2-	11+60	to	12+86	127
-Y3-	11+13	to	14+16	303
			Total =	2,445 (±0.5 miles)

PHYSIOGRAPHY AND GEOLOGY

Physiography and Geology

The project is located in the Coastal Plain Physiographic Province. Soils in this area generally consist of sedimentary sands, clays, and silts. Topography along the project corridor is nearly flat to gently sloping. Natural ground elevations range from 24± feet above sea level at the bottom of the stream bed at –L- station 24+50 to 44± feet above sea level along the near the existing intersection at Piney Green Road.

Soil and Rock Properties

Soils encountered along the project corridor are divided into four categories based on origin: roadway embankment, alluvial, undivided coastal plain, and Belgrade Formation.

Roadway embankment soils consisting of medium dense sand and gravel (A-1-b) was encountered along the existing Commerce Road shoulders and on Piney Green Road beneath the median. Roadway embankment soils ranged in thickness from 1± to 2± feet thick.

Alluvial soils were encountered in the vicinity of the proposed culvert location near station 22+00. These soils consist of very loose to loose sand and clayey sand (A-2-4, A-2-6) and soft to stiff silty clay with little organic material (A-7-6). Alluvial soils were encountered from the ground surface to a maximum depth of 15 feet below the ground surface at the borehole location and range in elevation from 13± feet to 28± feet above sea level. Test results showed that the cohesive alluvial sediments are comprised of 10% organic matter, and range in natural moisture content from 32% to 77%. Plasticity Indices (PI) within the cohesive sediments range from 12 to 18.

Undivided coastal plain (UCP) sediments were encountered throughout the project corridor at the surface and beneath roadway embankment and alluvial soils. These sediments were at least 6 feet thick where encountered, and are comprised of very loose to medium dense sand and clayey sand (A-2-6, A-2-4, A-3) and soft to medium stiff silt, clay, and sandy clay (A-4, A-6, A-7-6). The cohesive sediments ranged natural in moisture from 11% to 25%. Plasticity Indices ranged from 3 to 25.

Coastal Plain sediments from the Belgrade Formation were encountered beneath the undivided coastal plain and alluvial sediments where boreholes were advanced below elevation 20± feet above sea level. These sediments are comprised of very loose to medium dense sand and clayey sand (A-2-6, A-2-4, A-3), containing cemented shell fragments at some depth intervals. Where encountered, these sediments were at least 20± feet thick.

Ground Water

All SPT borings were left open for at least 24 hours to allow ground water levels within the borehole to equilibrate with the surrounding conditions. Ground water data were collected in November of 2017, during a time of normal precipitation. Ground water elevations generally varied with topography, and ranged in elevation from 27± feet to 37± feet above sea level. Ground water depths varied from 0.7 feet to greater than 6.0 feet below the ground surface at the borehole location.

Undisturbed Samples

An undisturbed thin wall Shelby tube sample was collected at the following location and submitted for testing. Test results show that soils at this location are saturated clayey sands (A-2-6), with a Plasticity Index of 15. Proposed construction in the vicinity of this location includes the placement of a culvert and embankment fills as high as ±13 feet.

Sample No.	Station	Depth (ft)	Test
ST-1	21+76 40' LT	8.0-10.0	CU, Consolidated
			Undrained

Culvert at -L- Sta. 22+07±

Natural ground elevations in the vicinity of the proposed culvert location range from 25± feet above sea level at the bottom of the stream bed to 41± feet above sea level along existing Commerce Rd. Borings completed at the proposed culvert location show approximately 2± to 4± feet of very soft, silty and sandy clay (A-6, A-7-6), containing 10% organic material with a moisture content ranging from 32% to 77%. The alluvial clay is underlain by 8± to 12± feet of very loose to loose, gray, alluvial sand and clayey sand (A-2-4, A-2-6) containing trace organic materials. Soils belonging to the Belgrade Formation were observed beneath the alluvial soils at elevations ranging from 13± to 18± feet above sea level. These very loose to very dense sands (A-2-4, A-3) were at least 20 feet thick and are assumed to extend below the elevations at which the borings were terminated.

Culvert at -L- Sta. 24+60±

Natural ground elevations in the vicinity of the proposed culvert at this location range from 24± feet at the bottom of the stream bed to 40± feet above sea level in the residential area east of the proposed culvert location. Borings at the proposed culvert location were advanced into Undivided Coastal Plain sediments consisting of loose clayey sands (A-2-6), and medium stiff clays (A-6) at the surface. There is assumed to be alluvial sediments within the stream channel, but none were encountered on the adjacent stream banks at CULV2_LT. At CULV2_RT the boring was advanced through 4± feet of medium stiff silty clay (A-7-6). The Undivided Coastal Plain and alluvial sediments are underlain by very loose to dense sands (A-2-4, A-3, A-1-b) belonging to the Belgrade Formation. The top of the Belgrade Formation was observed at elevations ranging from

20± to 22± feet above sea level. These soils were at least 30± feet thick and are assumed to extend below the elevations at which the borings were terminated.

Areas of Special Geotechnical Interest

The following sections contain cohesive soils which have the potential to cause embankment and or subgrade issues during construction. Soils with Plasticity Indices higher than 15 were encountered in the following sections

Alignment	Begin Station	End Station
-L-	11+80	19+75
-L-	21+00	22+20
-L-	24+50	24+70
-L-	25+72	31+94

Alluvial soils were encountered in the following sections.

Alignment	Station	End Station
-L-	21+00	22+41
-L-	24+49	24+72

Ponds occur in the following area within or near the proposed right of way.

Alignment	Station	Offset
-L-	20+00	±80' LT

Perennial streams were encountered at the locations below.

Alignment	Station	Offset
-L-	22+08	CL
-L-	24+56	CL

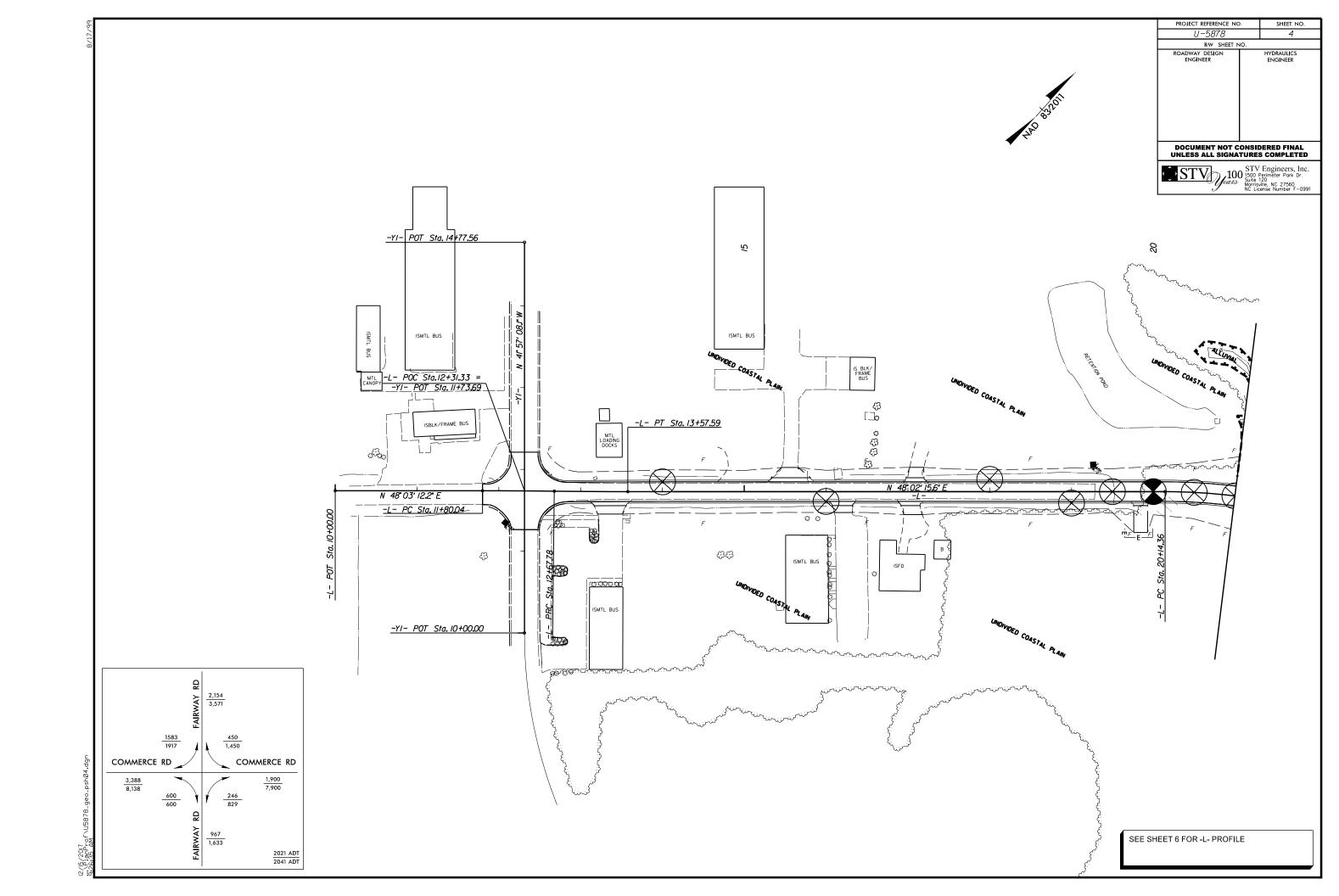
Sincerely, HDR ENGINEERING, INC.

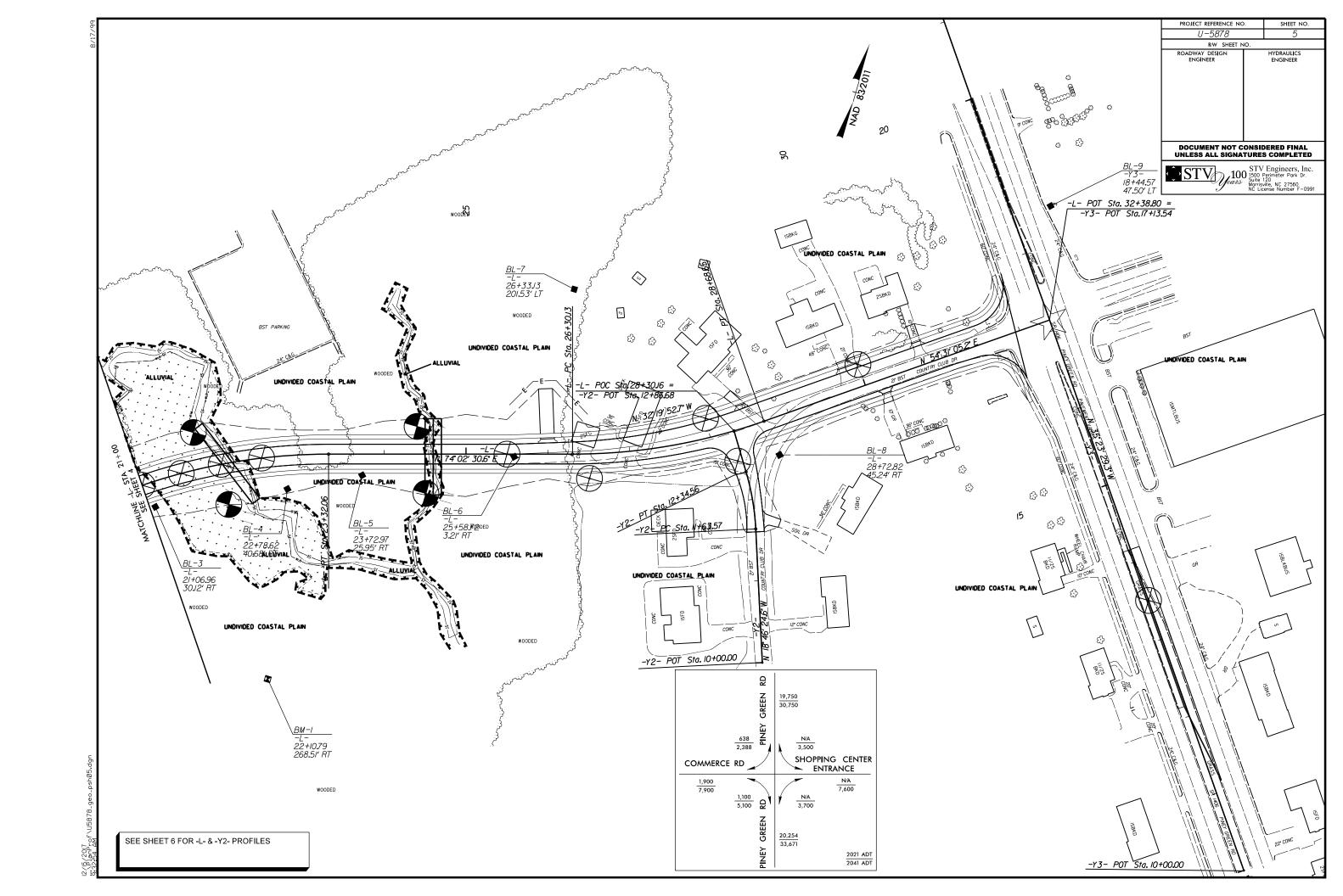


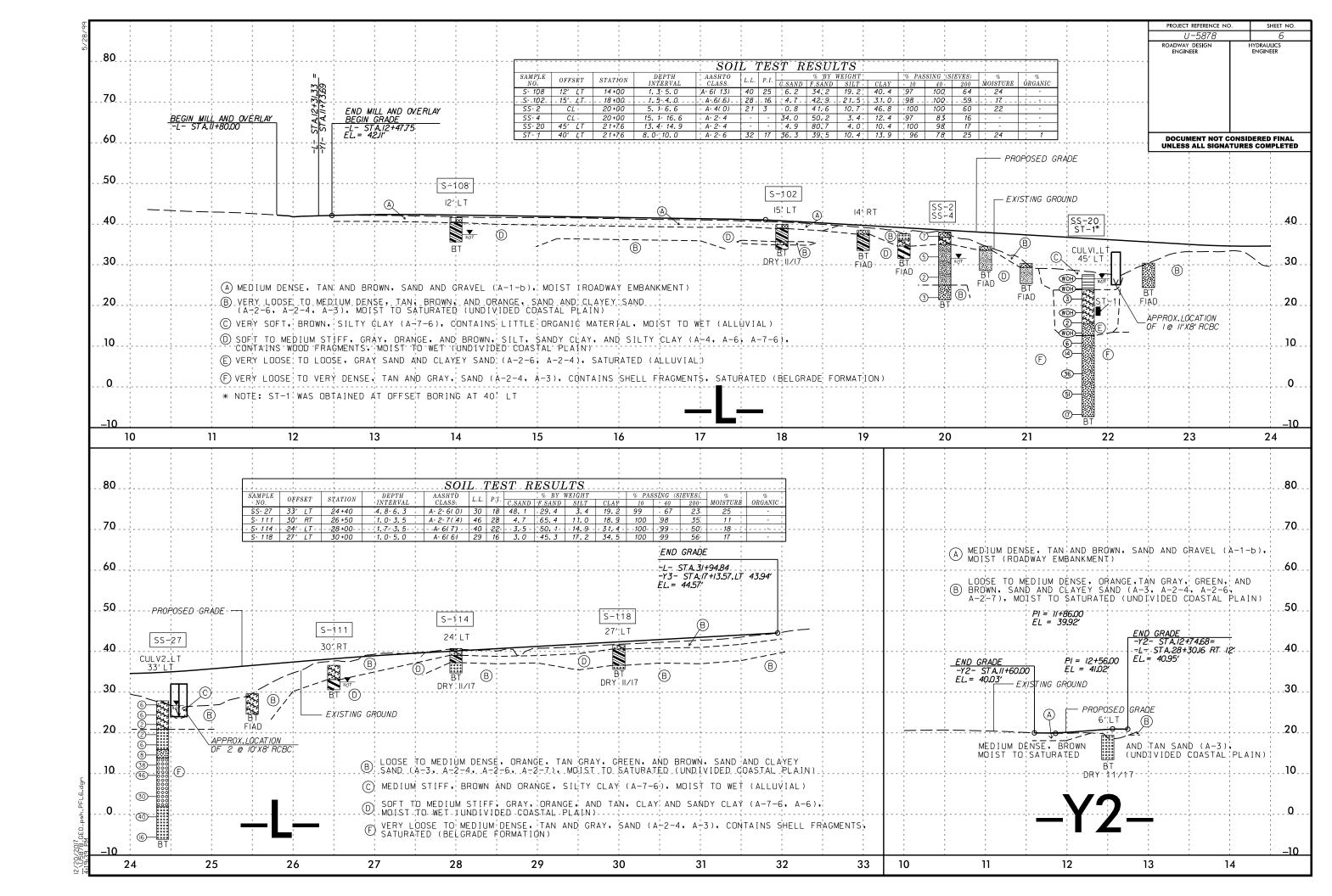
Jared K. Crenshaw, L.G. Professional Geologist



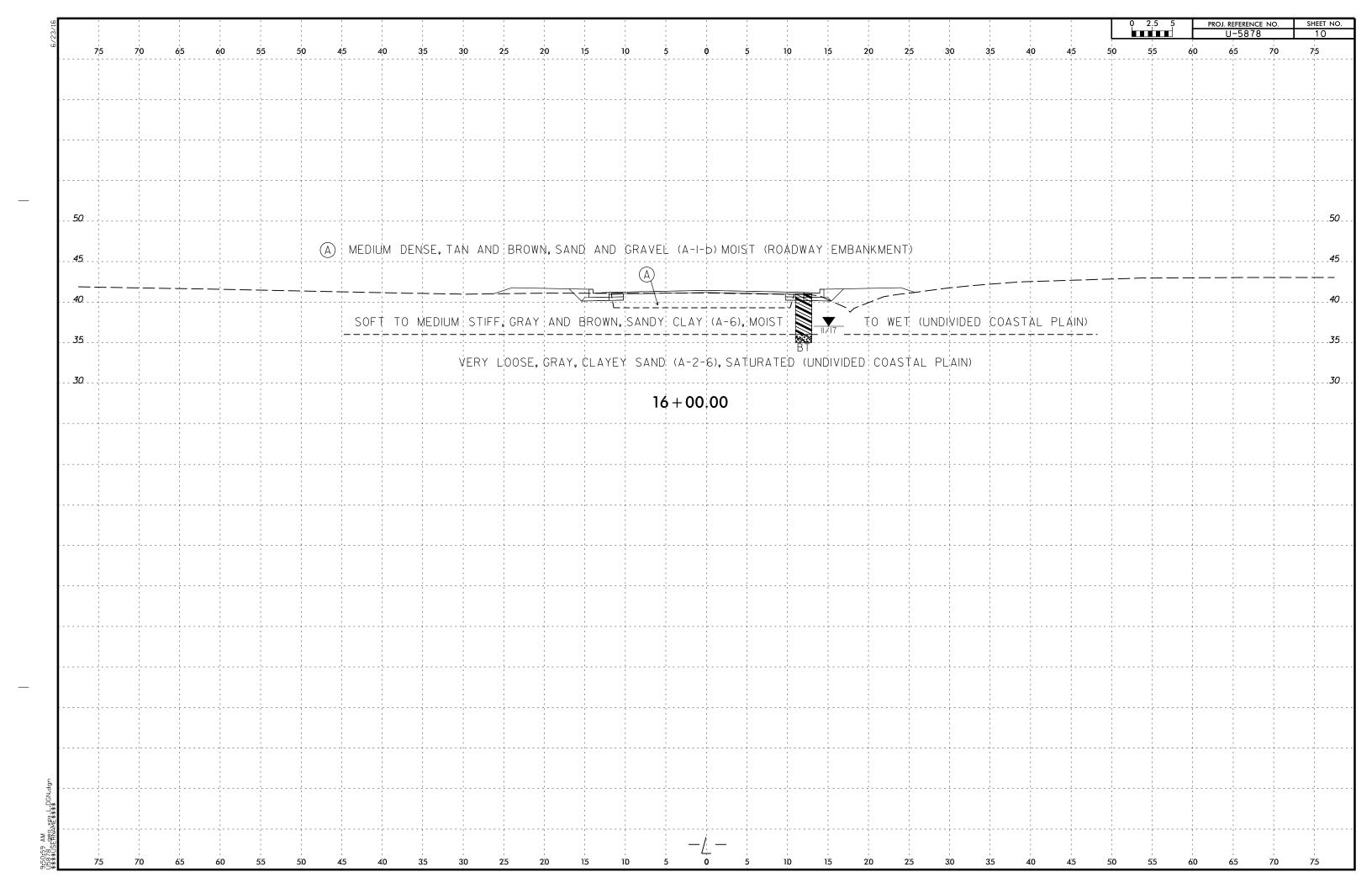
Brian Keaney, P.E. Project Manager

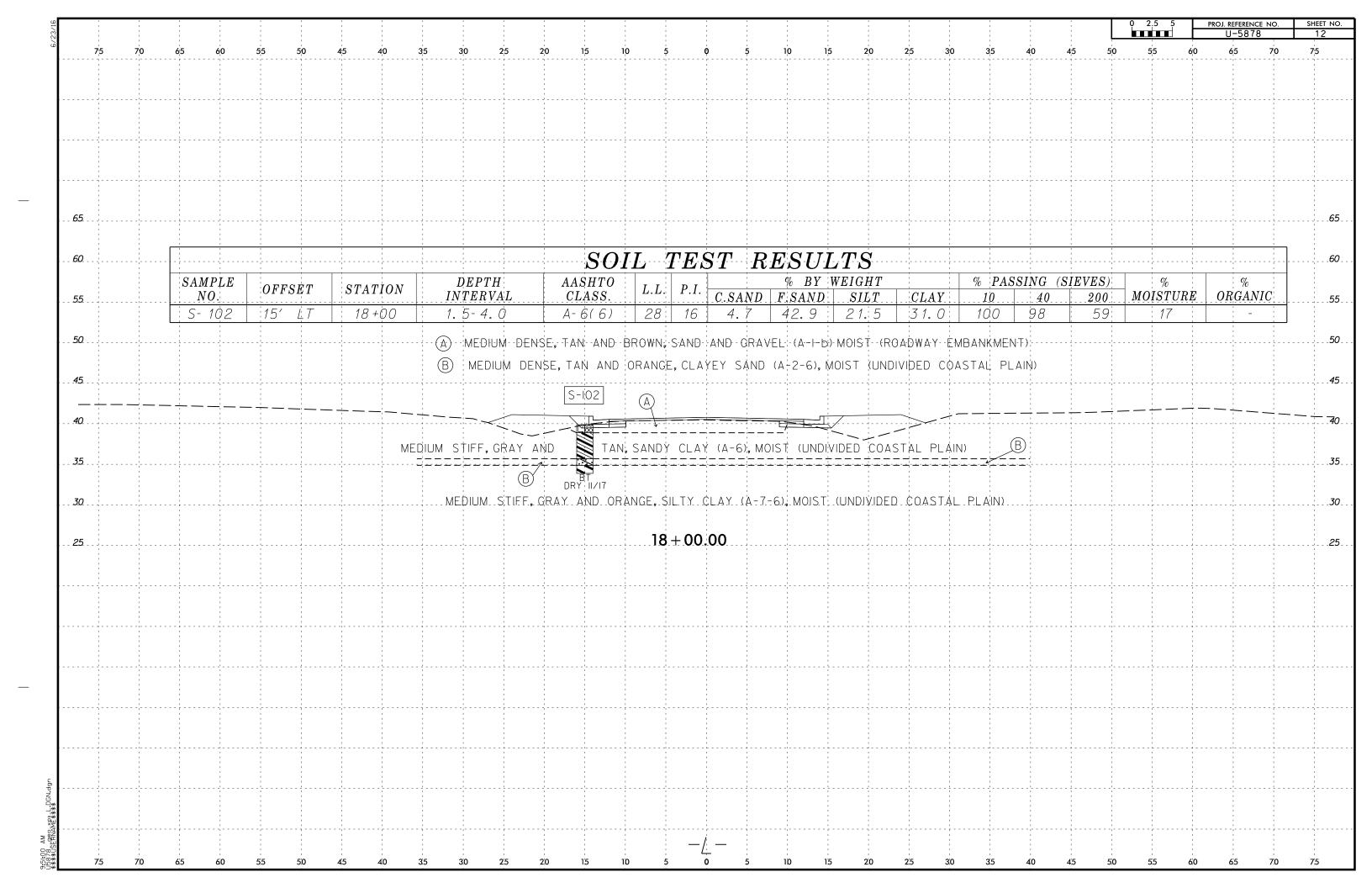


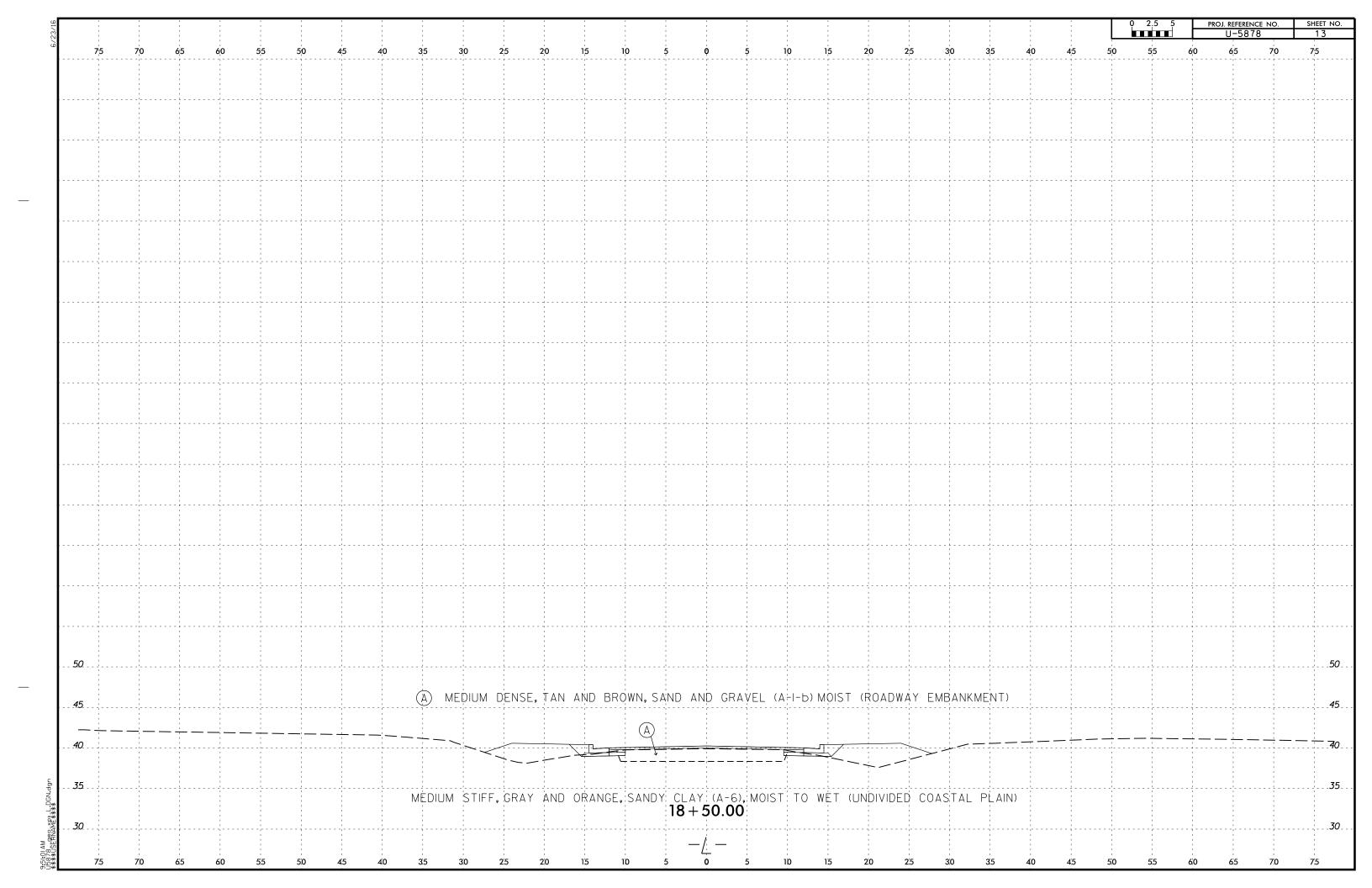


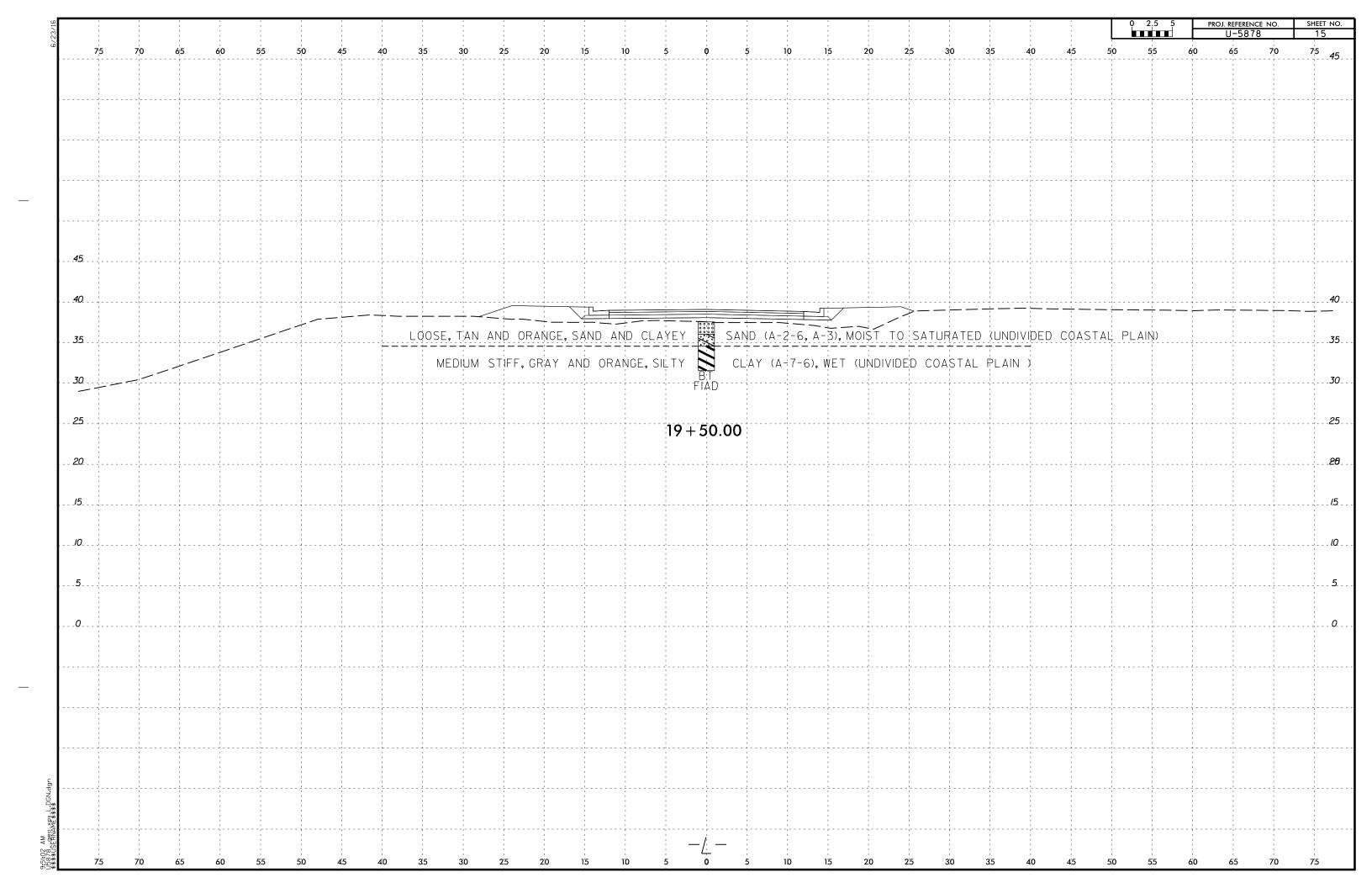


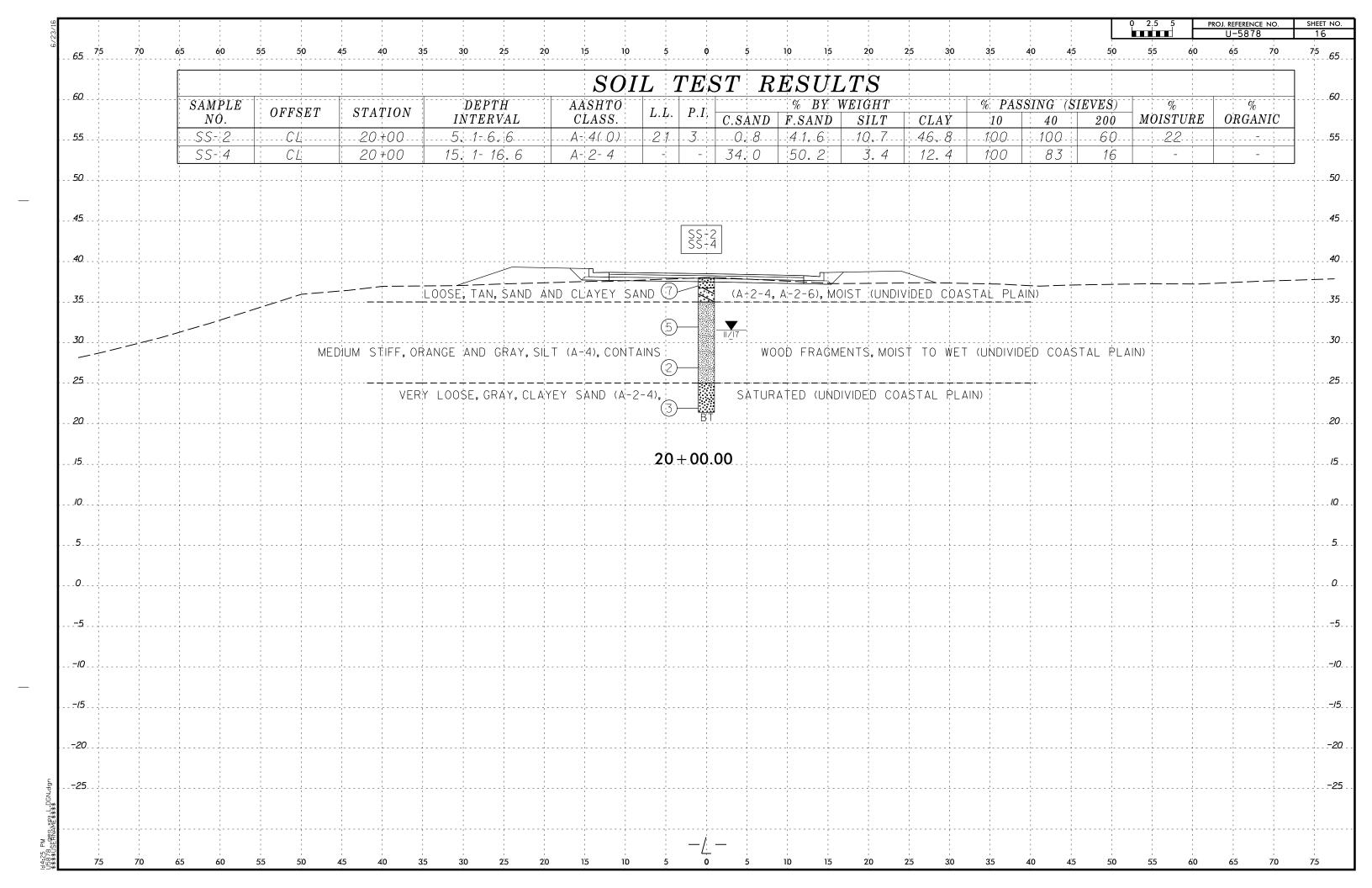
1	-	1	1		1 1 1	1 1 1	1 1 1 1	1 1 1 1	1 1 1	1 1			 	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	1 1 1 1		1	1		1 1 1 1			1	Į	0 2.5 5	PROJ. REFEREN	CE NO.	SHEE B
75	70	65	5 60	55	50	4	5 4	40	35	30 25	5 2	20 1	5	10	5	- - -	5	10	15	20	25	30 3	5 40	45	5(D 5/5 6	0 65	70	75
	1	1	1		1 1 1	1	! !	1	1			1	! !			1		1	1		1			1	1				
							!					<u>.</u>	!	<u> </u>															
						1	! !	1	1			1	! !			1			1										
							' ' !	<u> </u>	1	 					<u> </u>										· · · · · · .		 		
			1		1	1	 	1	1			ς							\overline{LTS}		1		1	1			1 1		
2		S 1	\overline{AMPLE}	:	1	- ;	1 1 1	<u>:</u>	, D E	EPTH		-AASH		Ii I	1	-	-	_	WEIGH	-			\overline{ASSING}	(SIFV	FS)		%		
·			NO.	OF	FSET	S	TATIO	N	INT	ERVAL		CLAS		L.L.	<i>P.I.</i>	C.SAN			SILT		\overline{CLAY}	10	40		200	MOISTURE	ORGANI	il .	
- !	1		5- 108	12	′ <u>L T:</u>	1	14+00	;		-5.0		A-6(1		: 40	25	6.2		4. 3	19. 4		40.4	100			64	24	-		
									1					<u> </u>	:			!		 								!	1
1 1 1	1	1	1 1		1 1 1	1	1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1				1 1 1	1		1 1 1	1	1		1	1 1 1		1	1	1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	
!					1	:	 	<u> </u>		-¦	DENCE										DWAY 5		<u>-</u>						
1	1	1					! !	1	(A) N	/EDIUM :	DENSE	I AN	AND B	ROWN,	SAND .	AND GF	YA VEL	(A-I-D	MOIST	RUA	DWAY E	MBANKM	ENI)						
i										-			-5-108	3				-			·						i 		
					1	1	, , ,		1							1						<u> </u>		- — — -					_
		-											- 1	1	-			<u> </u>											
			1		1		· ·		1												1		1						
						,	! ! !	¦ 				¦ 		/ 7 				-									, , , , , , , , , , , , , , , , , , , ,		
1			1		1	1	! ! !	505	; :'T T	MEDILIM	C T I E I		BT	; RDOWN	; SVID	; \\	; V (A-6) MOIS	; T TO 1	Wet (: UŅDIVIDE		; Etal Di	A INI)	į		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1
						 - 	 	. 301	1 10		ا ۱۱۱ د	;, GIVA I		LINOWN	-¦	L CLA	i (A 0	ا ۱۷۱ و ۱۷۱ ا ا		VV:∟ I (¦	LD COAS		_ AIIN/			! ! !		! ! !
1 1	1	1	1		1	1 1 1	1 1 1	1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			1 1 1	1 1 1	1	111	00.00	i	1			1		! ! !	1	1		! ! ! ! ! ! ! ! ! ! ! ! ! ! ! ! ! ! !		1
<u>.</u>							' ' '					}	' ') 14 T		<u> </u>												
1 1 1		1			1	1 1 1	 	1 1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	 	1		1 1 1		1			1		1		!		1 1		1
<u>.</u>	1	1		-	1	1 1 1	1 1 1 1	1 1 1	1 1 1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1 1	1		1 1 1	1	1 1 1	1 1 1 1		1		1		1				1
			 				, , ,		1	1 1		1	,	1	1	-					. = -j= = = = = = = = = = = = = = = = = =	·			1 1		1		
1		1	1		1 1 1	1	1 1 1	1 1 1	1 1 1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	 	1		1 1 1	1	1	1	1	1		1		1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1
		<u>-</u>			<u>-</u>	! ! !	' ' '		1	- (J	' ' '	1		- L											J		
1	1	1	1 1 1		1 1 1	! ! !	1 1 1	1 1 1	1 1 1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	! ! !	1		1 1 1	1	1	1	1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1	1	1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	1
	¦				1 1	₋ , , ,	 	<u>-</u>	1				 	1		- 			- 1			-					· · · · · · · · · · · · · · · · · · ·		:
1	1	1	1		1 1 1	1	1 1 1	1 1 1	1 1 1			1 1 1	1 1 1	1		1 1 1	1	1	1	1	1		1	1	1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1
						1 1			1			1															· · · · · · · · · · · · · · · · · · ·		
1		1	1		1 1 1	1	1 1 1	1 1 1	1 1 1			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1 1 1	1		1 1 1	1	1		1	1		1 1 1	1	1		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1
					<u>1</u>		'	<u>.</u>	1 2) 	'	1							!						<u> </u>		
1		1	!		1	1	1 1 1	1 1 1	1			1	 			1		1			1		1	1					1
					 		 - 		1			<u> </u>	 	† 															
						1		1	1			1	 			1		1											1
				<u>.</u> .			!																				i 		
1			1		1 1 1	1 1 1	! !	1	1			· !				1	1		1				!	1 1	1				
						ا ا ا	: : : '	 - 	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2			 - 		 	1	 - 		 									! ! !		
1 1		1	1		1 1 1	- 1 1		1	1 1 1		_					1			1		1		1	1 1 1	1			1 1	
1 1 1	1	1	1 1 1		1 1 1	1	1 1 1	1 1 1	1 1 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1 1 1	 	1		1 1 1	1	1	1 1 1	1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1	1 1 1	1 1 1	1	1 I I I I I I I I I I I I I I I I I I I	1 1	1
					1		, ! !		1			,	,	T	<u> </u>	/ _		1			,			· · · · · · · · · · · · · · · · · · ·			· · · · · · · · · · · · · · · · · · ·		
1	i		- 40	-			!	1			_	1		10		<u>_</u>	1	-1-	1				- 10		-		1 1	;	

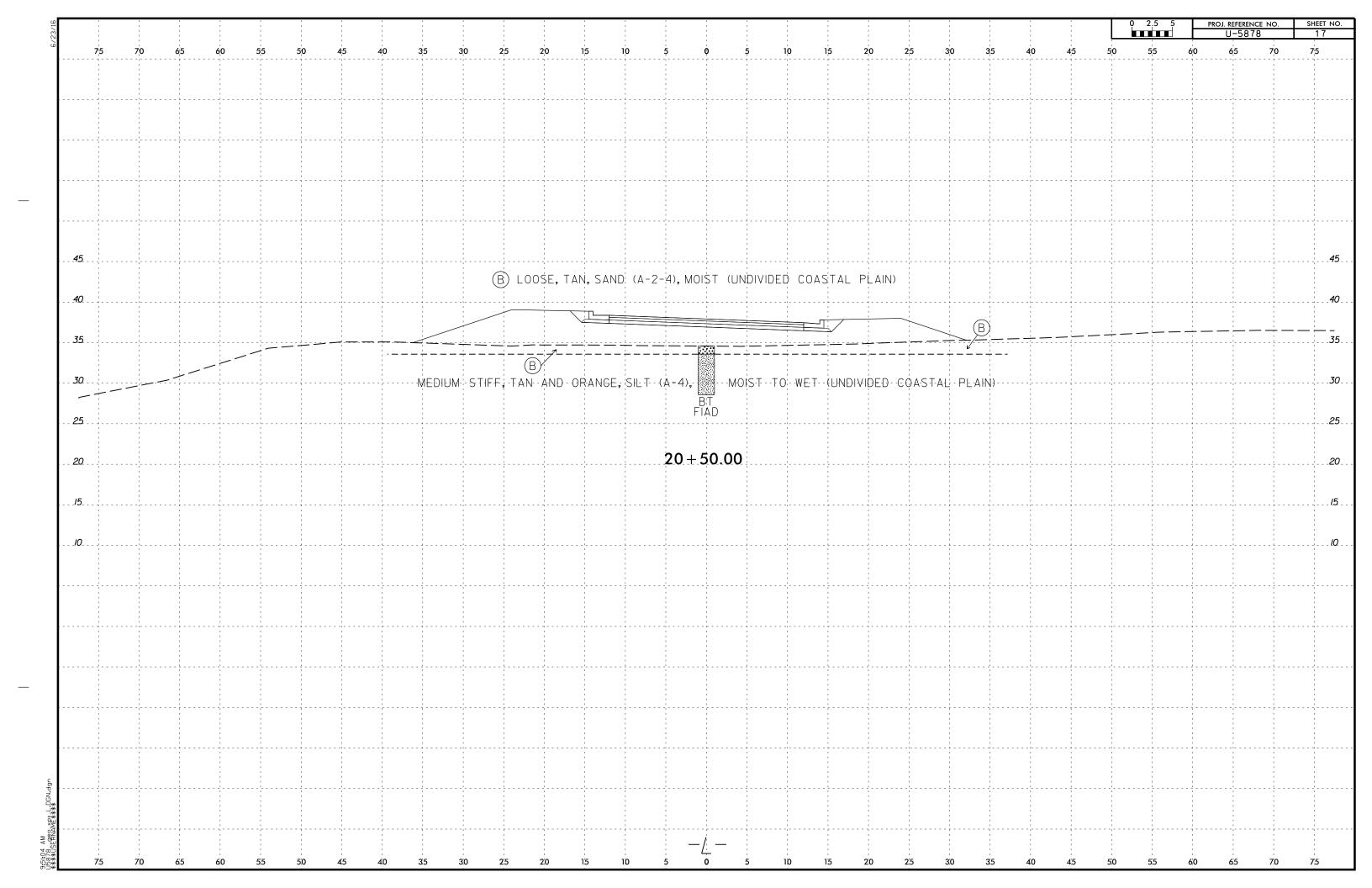


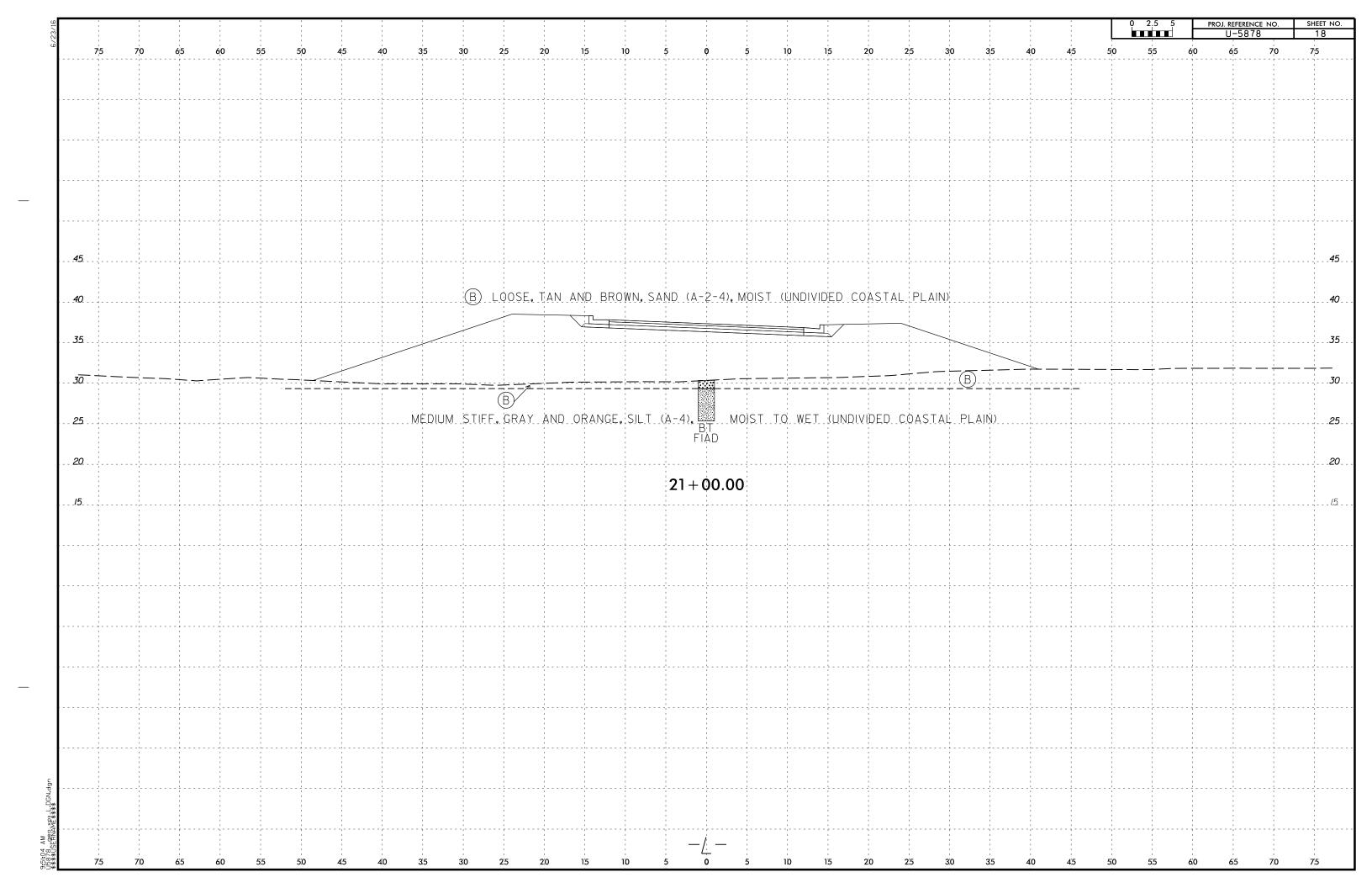


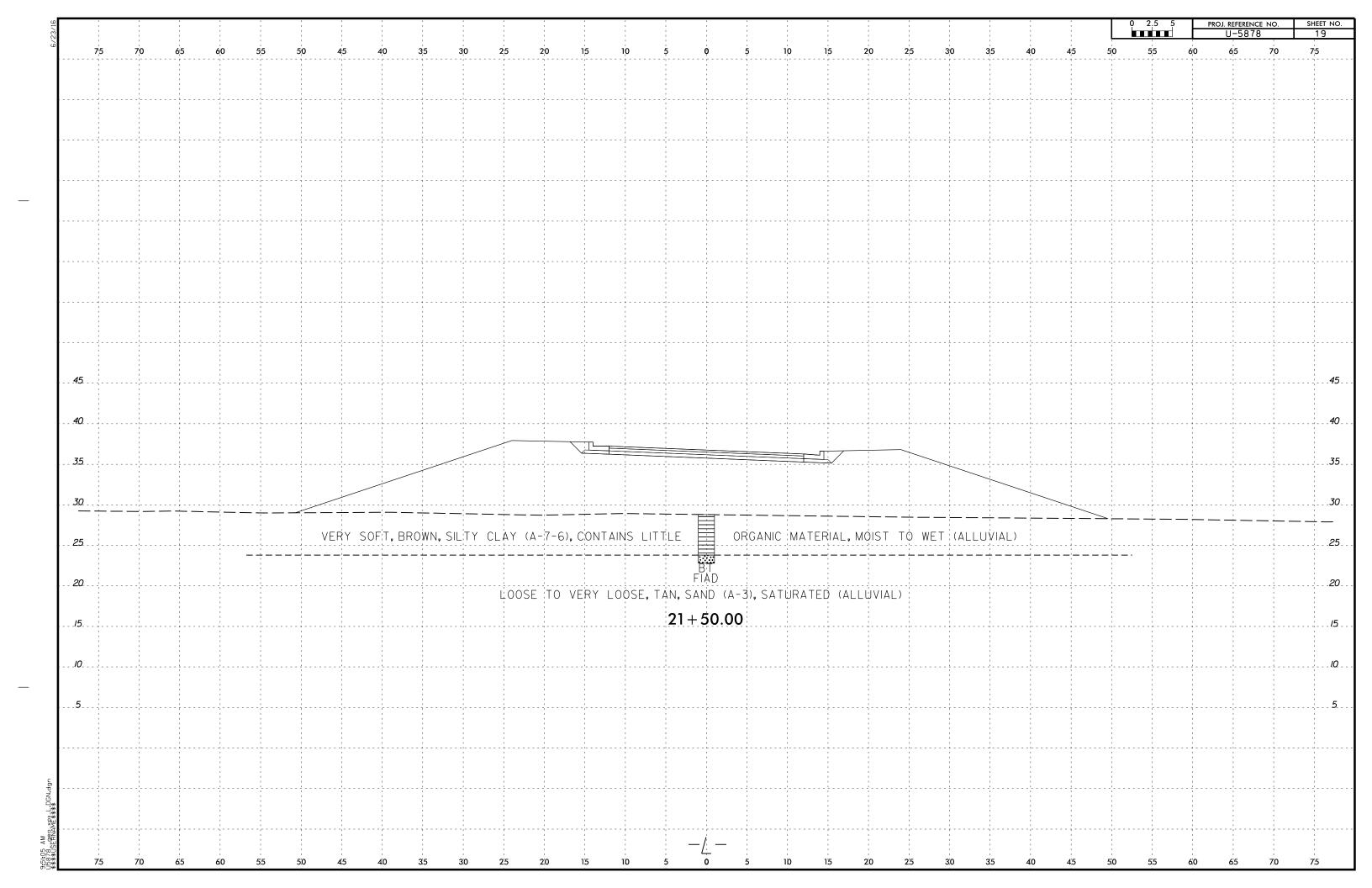


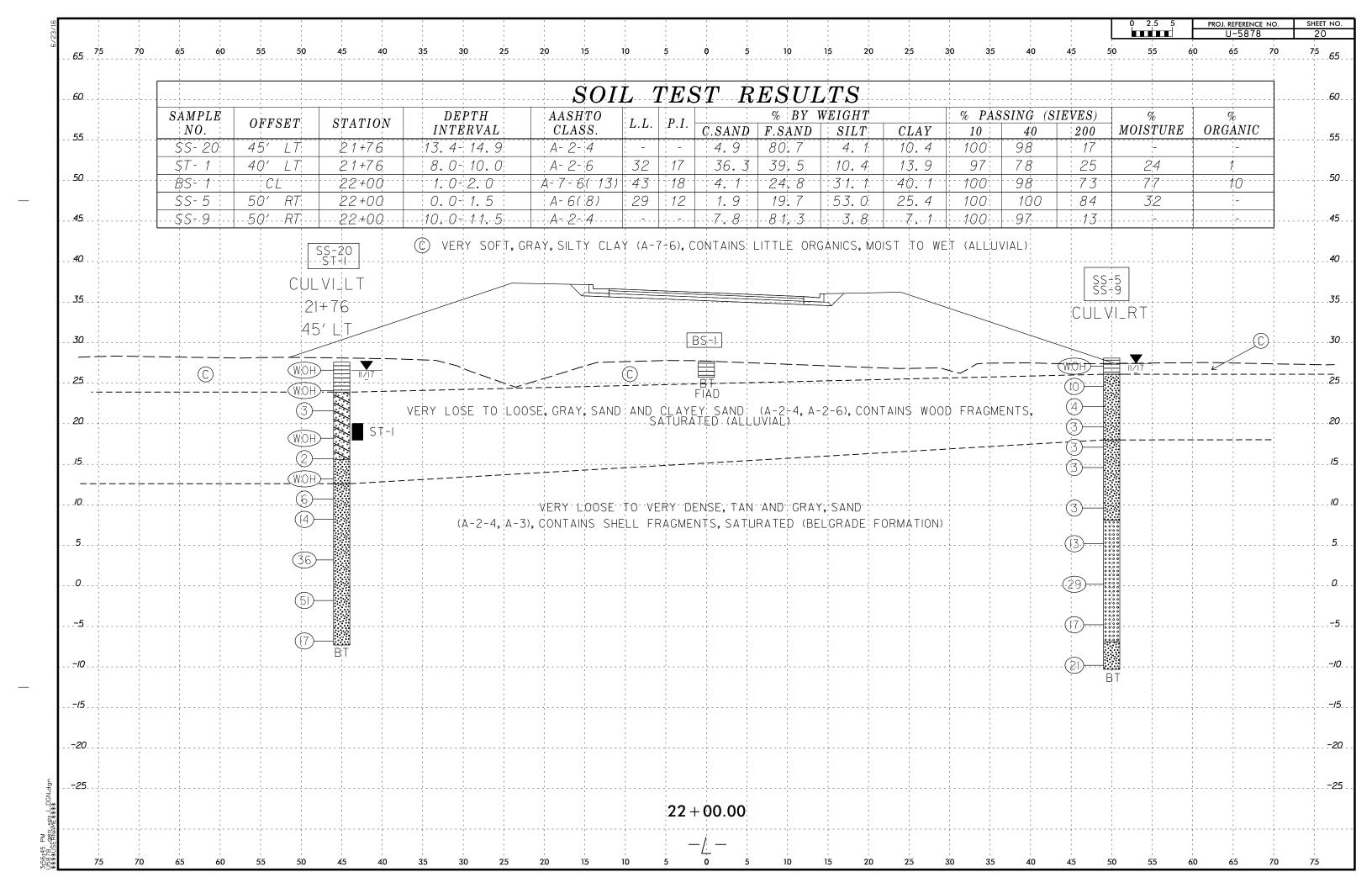


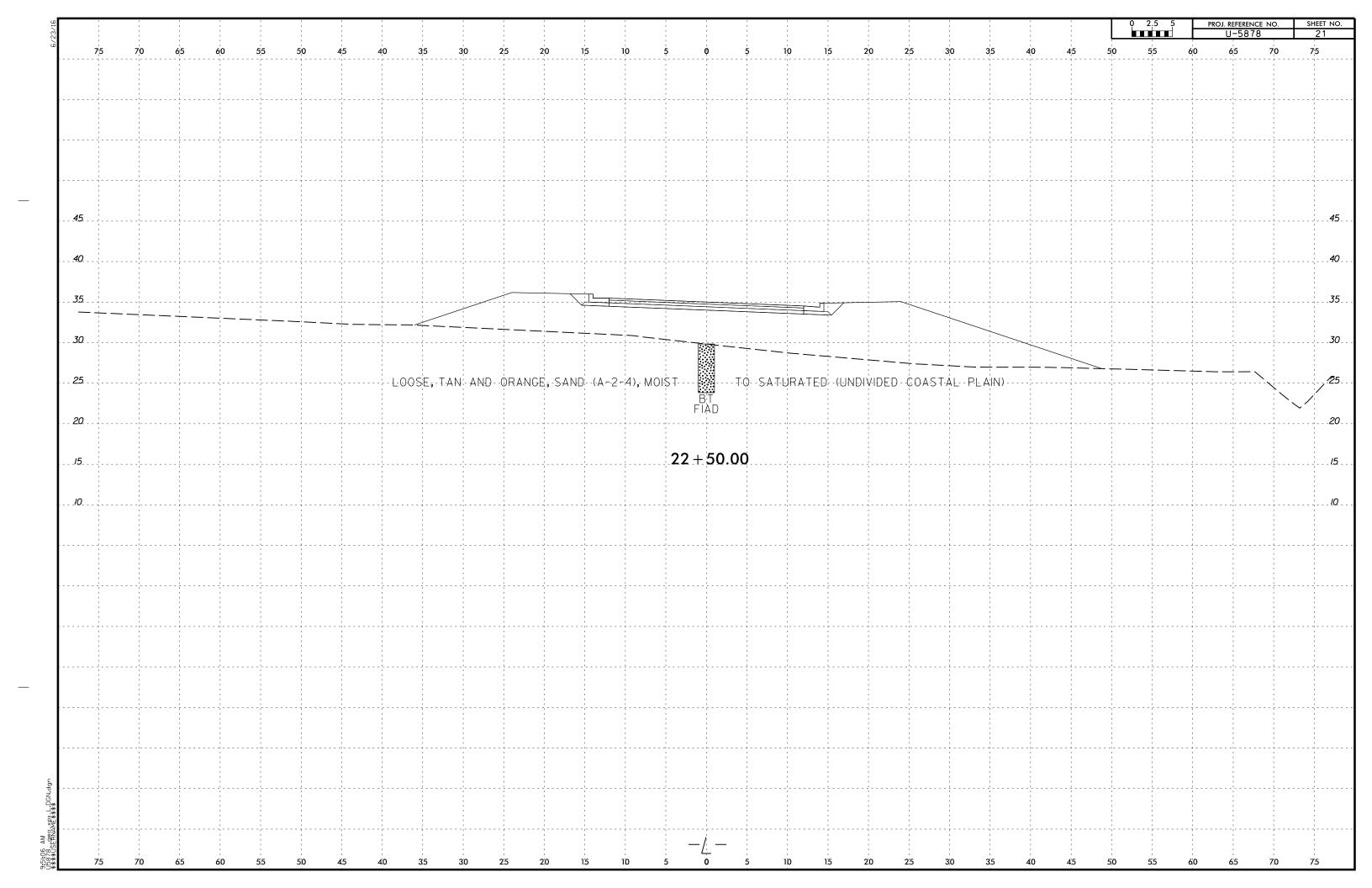


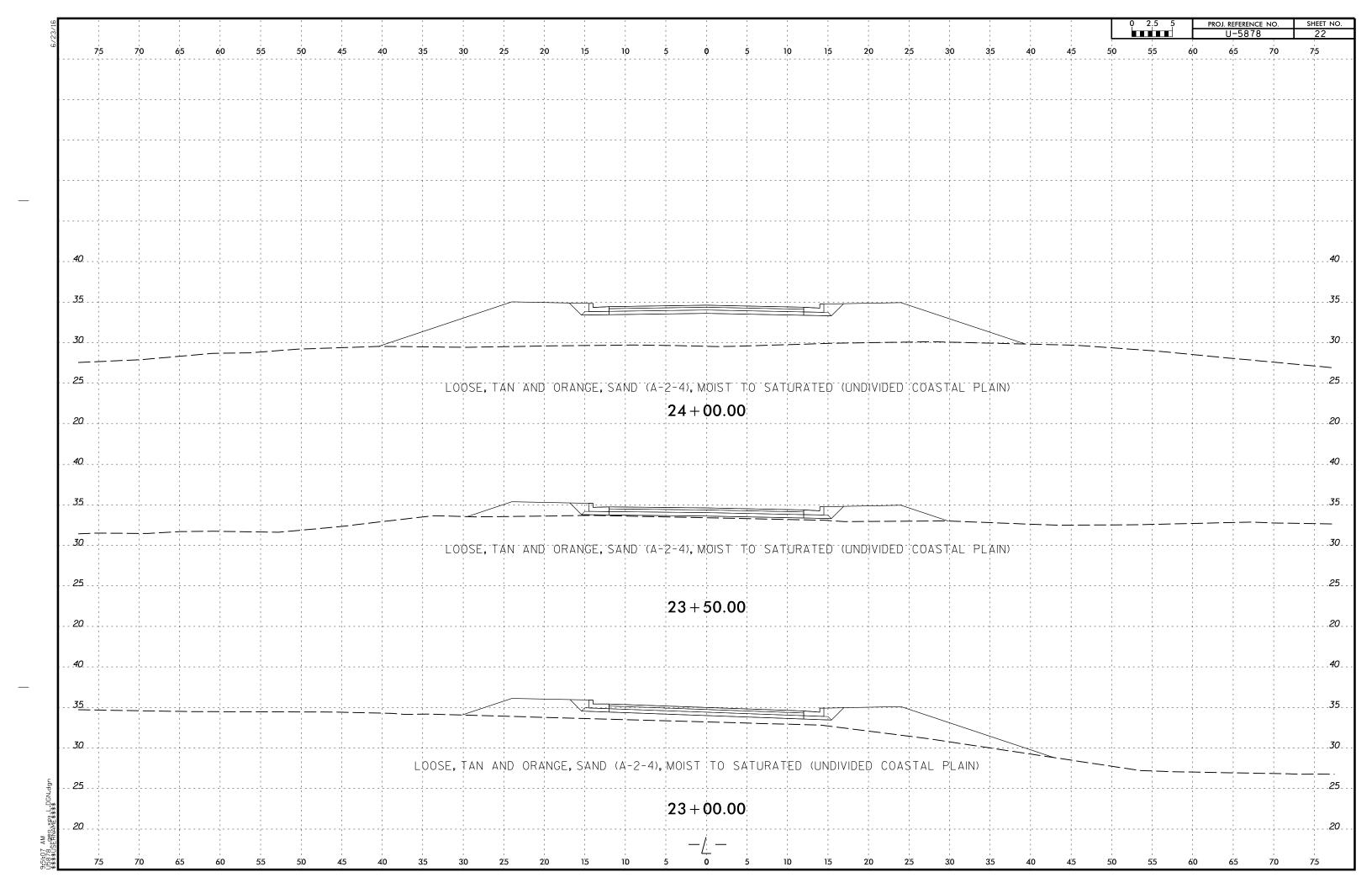


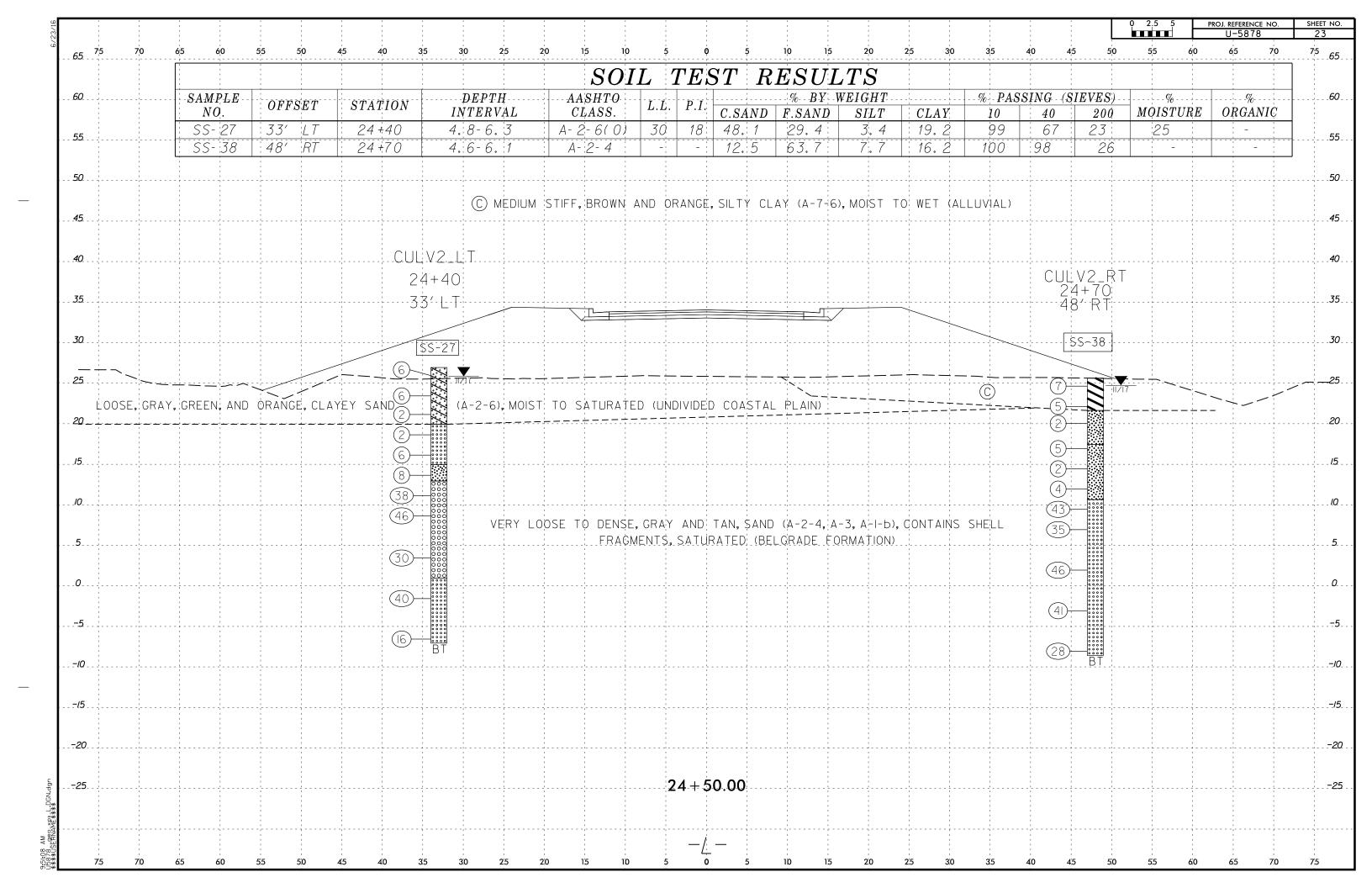


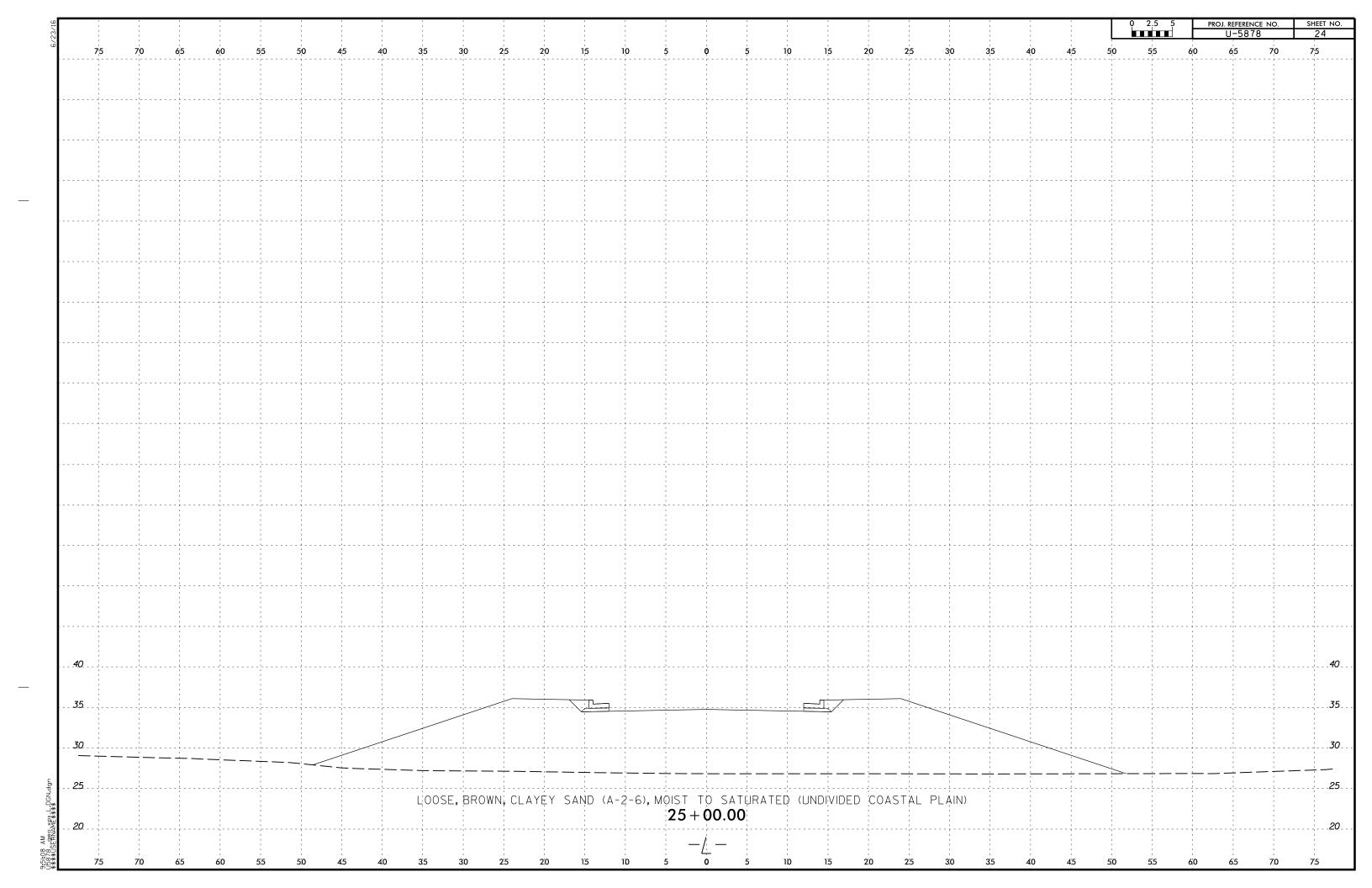


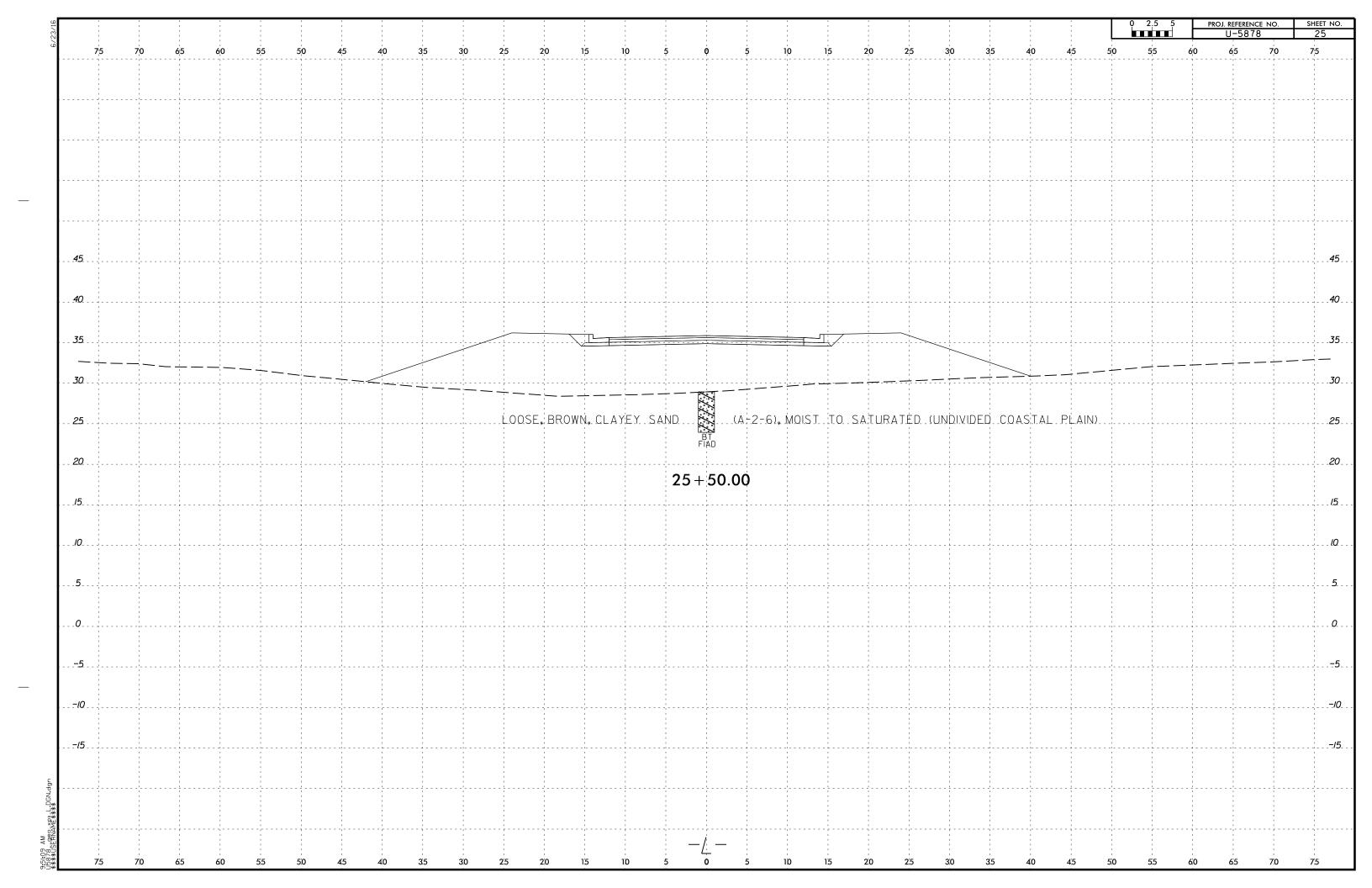


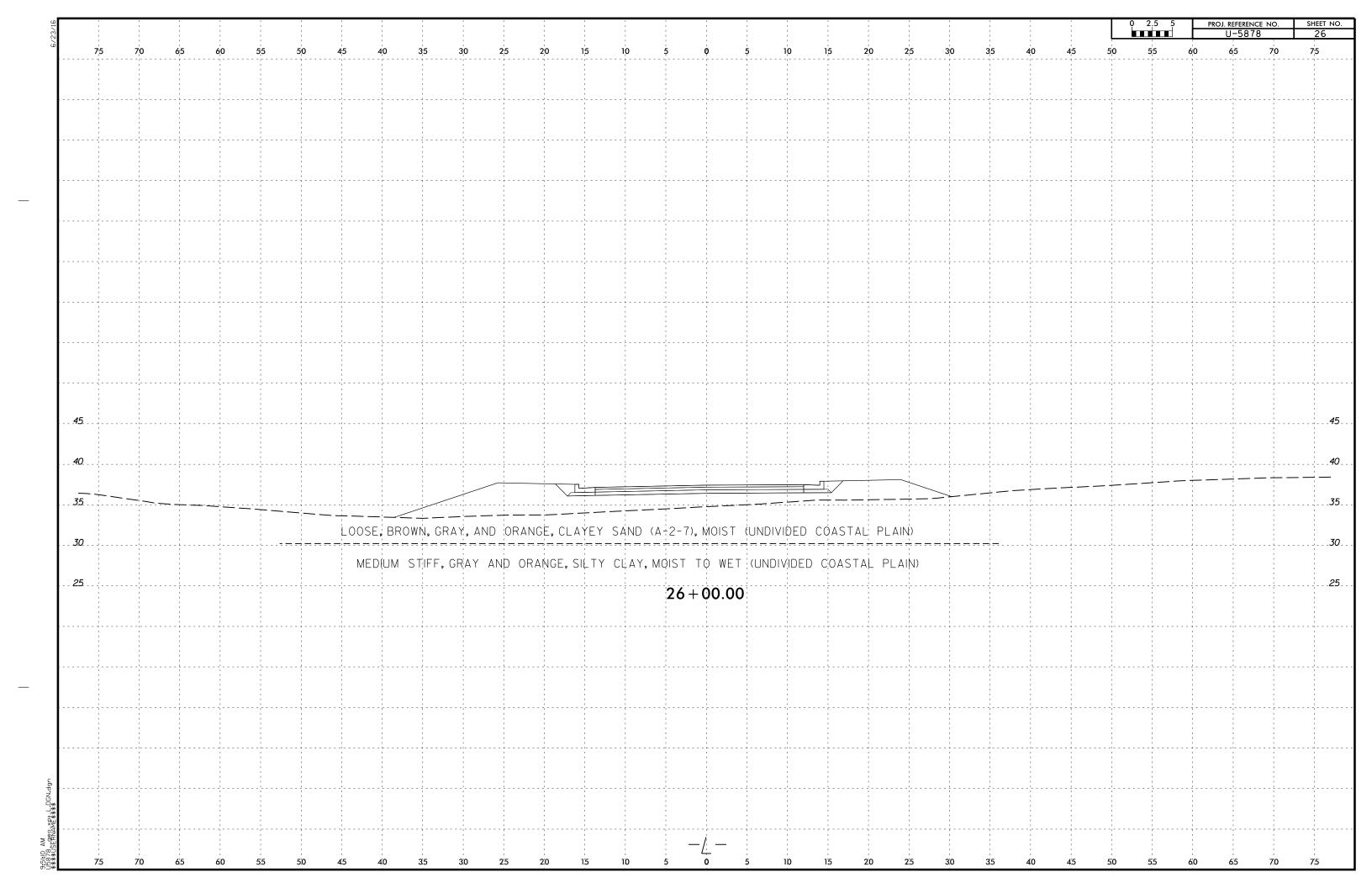


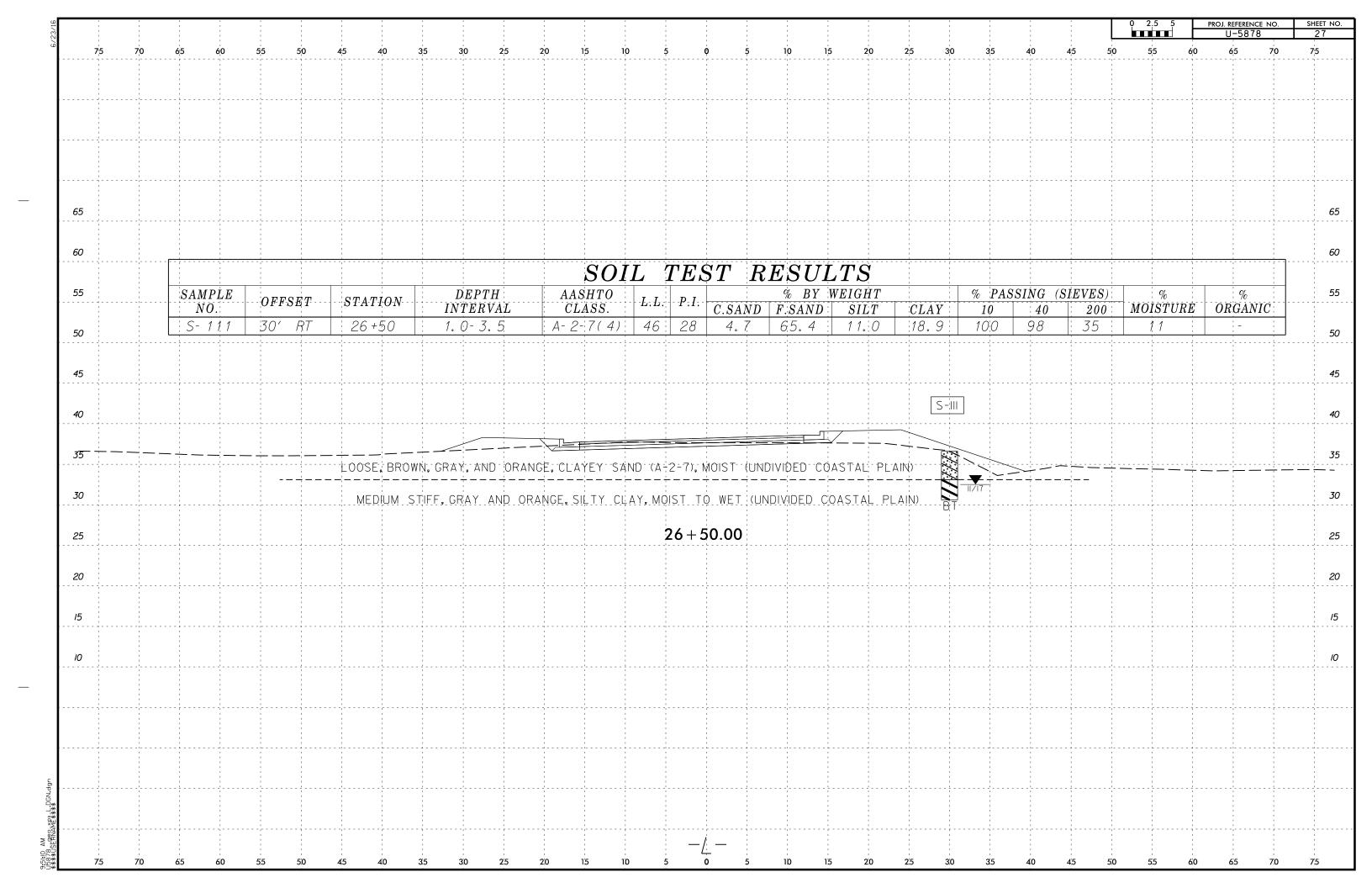


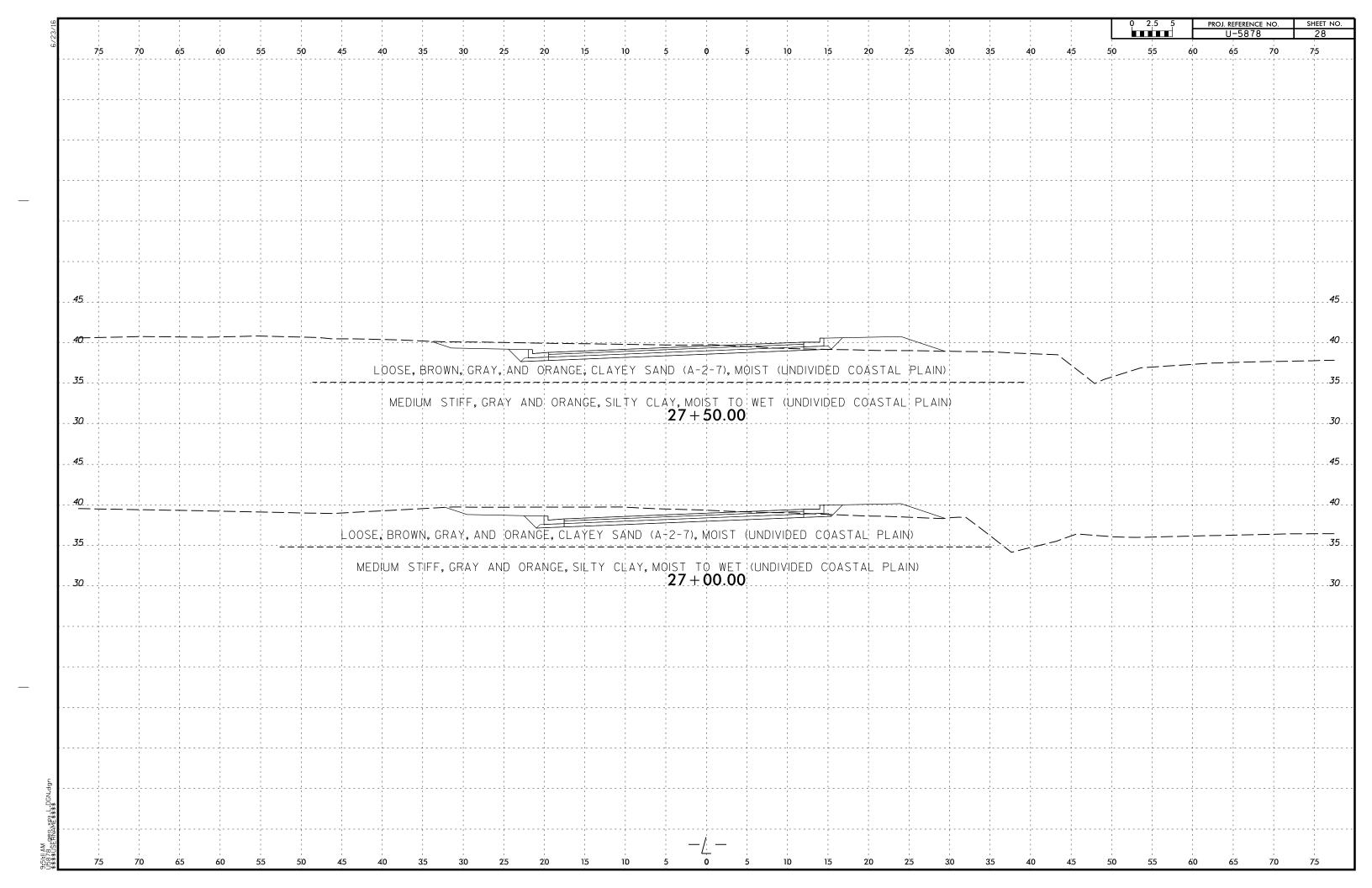


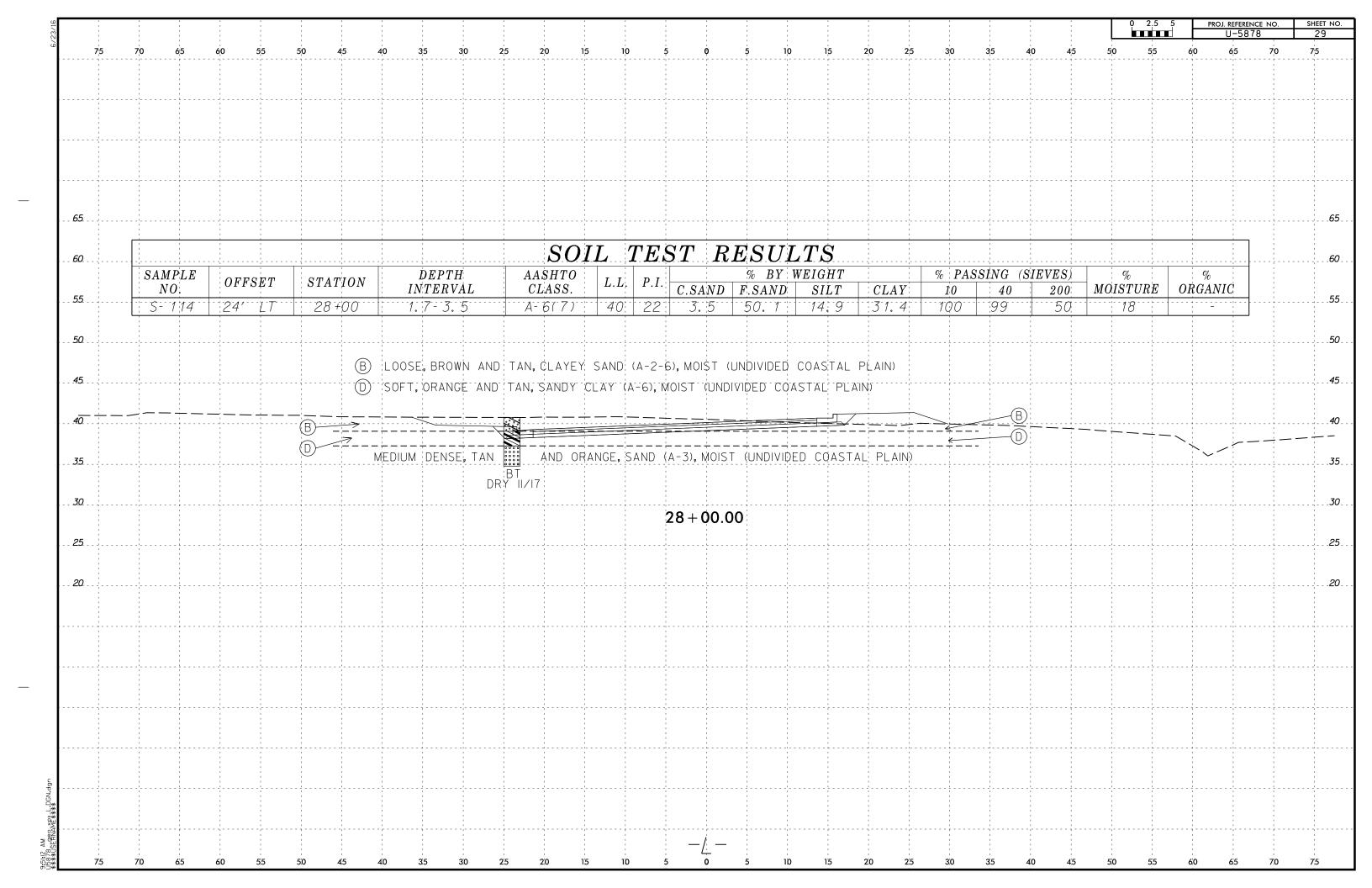


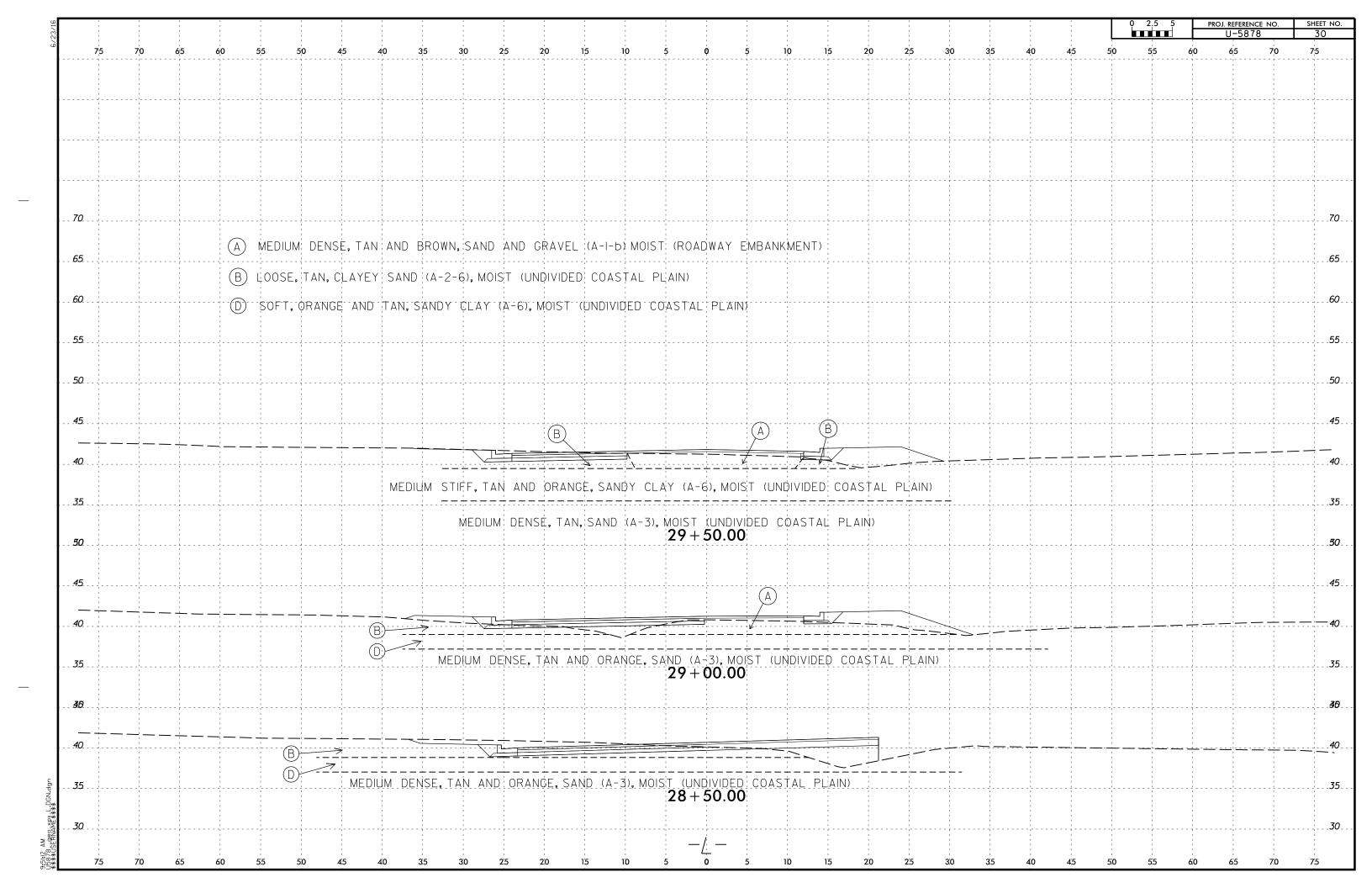


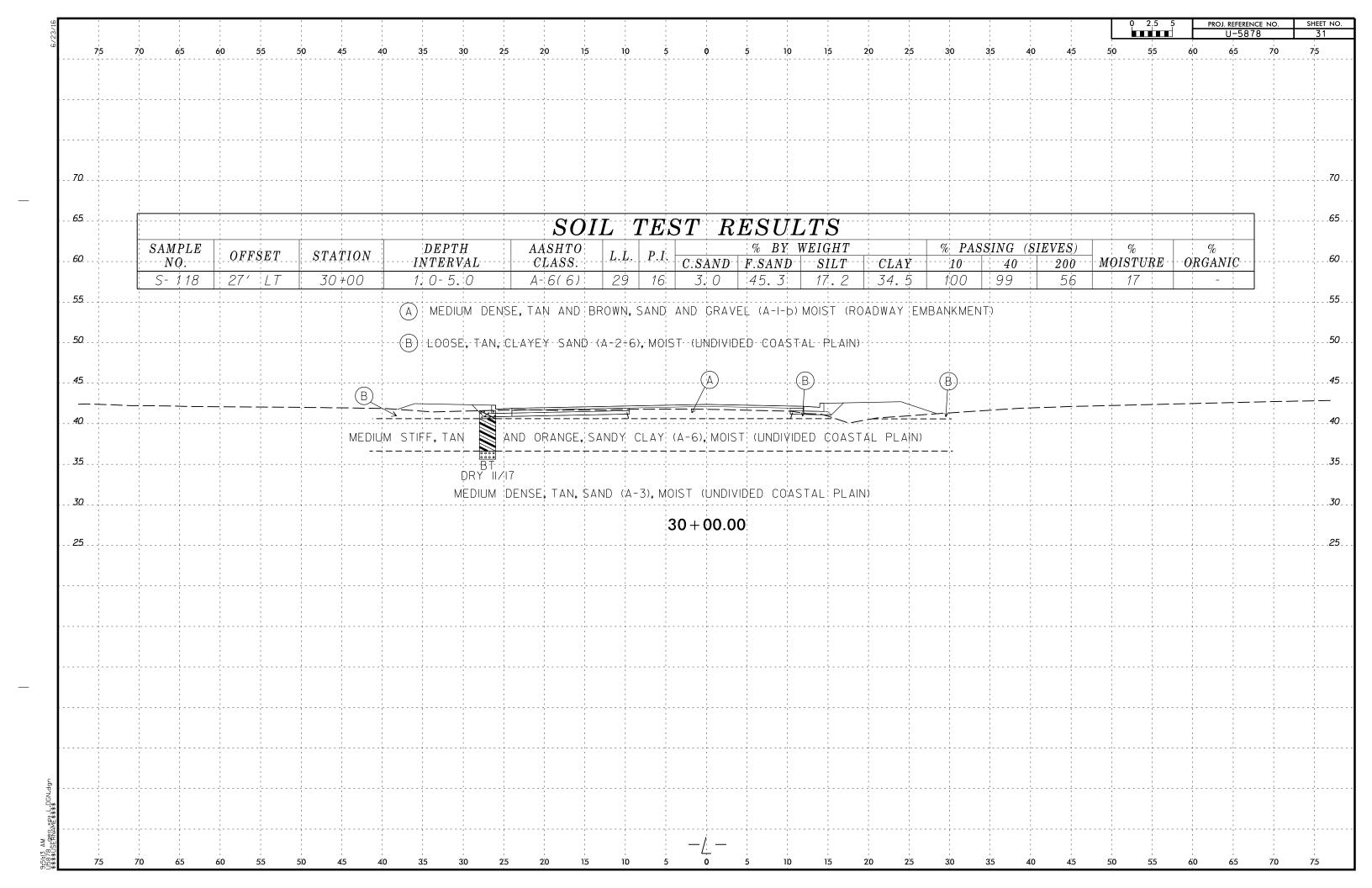












		0 2.5 5	PROJ. REFERENCE NO. U-5878	SHE
75 70	65 60 55 50 45 40 35 30 25 20 15 10 5 0 5 10 15 20 25 30 35 40 45	50 55	60 65 70	7,5
				1
				1
				;
	A MEDIUM DENGE TAN AND PROWN CAND AND CRAVEL (ALLEY MOIST (DOADWAY EMPANICATION)			
, , , , , , , , , , , , , , , , , , ,	A MEDIUM DENSE, TAN AND BROWN, SAND AND GRAVEL (A-I-b) MOIST (ROADWAY EMBANKMENT)			
2				
_				
i				
)				
1 1		1 1		
i	eta			
_ 				- — —
2	MEDIUM STIFF, TAN AND ORANGE, SANDY CLAY (A-6), MOIST (UNDIVIDED COASTAL PLAIN)			
i 	MEDIUM DENSE, TAN, SAND (A+3), MOIST (UNDIVIDED COASTAL PLAIN) 31+50.00			
				! ! !
5				
1	(B) LOOSE, TAN, CLAYEY SAND (A-2+6), MOIST (UNDIVIDED COASTAL PLAIN)			
i				
_	B)		<u> </u>	
2	ACTIFE TAN AND ORANGE CANDY OF AN AND ORANGE CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR AND ORDER CANDY OR AND ORDER CANDY OR AND ORDER CANDY OR AND OR			
	MEDIUM STIFF, TAN AND ORANGE, SANDY CLAY (A-6), MOIST (UNDIVIDED COASTAL PLAIN)			
5	MEDIUM DENSE, TAN, SAND (A-3), MOIST (UNDIVIDED COASTAL PLAIN) 31+00.00			
	31+00.00			
2				
<u>.</u>	\widehat{A} \widehat{B}			
	B			:
	MEDIUM STIFF, TAN AND ORANGE, SANDY CLAY (A-6), MOIST (UNDIVIDED COASTAL PLAIN)			
5	MEDIUM DENSE, TAN, SAND (A-3), MOIST (UNDIVIDED COASTAL PLAIN)			
	30 + 50.00			
		1 1	The second secon	1

